

UTDOT RESEARCH



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COLD IN-PLACE RECYCLE PHASE III: MIX DESIGN

Prepared For:

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16. Abstract This project's purpose is to revise the UDOT accepted design methods for Cold In-Place Recycling so that they better reflect field behavior and target the desirable attributes of the material. The previous design process failed to adequately predict behavior related to temperature as well as did a poor job of characterizing return to traffic performance. This project calls upon both laboratory observation and field experience to obtain optimization between rutting, stripping and intermediate temperature cracking. It also ties down the constructability and return to traffic parameters required by maintenance of traffic constraints. The new design process continues the use of Marshall Stability and Tensile Strength Ratio to set rutting and stripping resistance. It further uses Semicircular bending to set an intermediate temperature cracking parameter. New compaction/temperature and compaction/gradation relationships are developed to help determine final density and emulsion targets. A new test for emulsion set time is developed to prequalify emulsion construction performance. Modified BBR and Elastic Recovery testing is used to establish cold temperature properties of recovered binder.			
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LIST OF ACRONYMS

AASHTO	American Association of State Highway and Transportation Officials
CIR	Cold In-Place Recycled asphalt
DCT	Disk-Shaped Compact Tensile Test
ESCRE	Engineered Solventless Cold Recycle Emulsion: Asphalt emulsion engineered to meet the requirements of CIR using less than 1% solvent.
FHWA	Federal Highway Administration
HMA	Hot Mix Asphalt
RAP	Recycled Asphalt Pavement
SCB	Semi-Circular Bending Test
SGC	Superpave Gyratory Compactor
UDOT	Utah Department of Transportation

EXECUTIVE SUMMARY

CIR has proven to be an efficient, economical tool in the pavement preservation tool box. CIR may be used under a chip seal on low volume roads, or as a base isolation interlayer in heavier traffic applications. Interspersed with great performance are those projects which underperform. This happens often enough to cause UDOT to want a more full understanding of the design and application of the product. Current mix design practices appear to have limitations as they relate to both material qualification and construction. Applicators regularly reduce the emulsion rate between 0.5 and 1.0 percent from the mix design target to prevent rutting and shoving. This behavior indicates a disconnect between laboratory design intentions and field compaction efforts. Further, no method exists to separate emulsions that are suitable for use in CIR and those which will not meet Department expectations, most specifically related to quick return of the pavement to traffic.

The expectations of a CIR project have always been that CIR is like Hot Mix Asphalt. One of the lessons from this research is that it is a very different material from HMA. It has markedly different strength, cracking potential and construction characteristics. Curing time must be controllable for aggressive maintenance of traffic to be managed. One of the goals of this research is to measure these properties and to determine appropriate applications for it. Another goal is to develop methods to obtain consistent results so that when pavement design is done, the as-constructed product will both carry the design loads, and perform adequately in an environmental durability category.

A method to determine temperature and gradation sensitivity is developed by using the superpave gyratory compactor to measure compaction variability. This technique is used to set target densities and emulsion contents at varying temperatures for varying gradation types. Field measurements of these two factors can then be used to manage optimum construction practices.

The use of the Semi-Circular Bending test is investigated in developing standards for cracking behavior. It is unknown at present whether high fracture toughness and fracture energy are desired in a base isolation interlayer.

A method to qualify emulsions for CIR use is also developed allowing the Department to determine, in advance, the demulsification rates of various asphalt emulsions. This is critical to

the support of CIR by UDOT as the department has set a premium on limiting traffic impacts. It is also critical to the evaluation of emulsions for constructability as the potential for aggressive emulsions to flash-set in the CIR train will increase. A set of proposed specification standards are proposed.

1.0 INTRODUCTION

1.1 Problem Statement

As asphalt roadways become more expensive and less lane miles can be maintained, UDOT is exploring methods to effectively rehabilitate pavements at lower cost. Cold In-Place recycle has been shown to provide a strong base layer allowing construction of a thinner load distribution layer. CIR has also been shown to do a good job of strengthening a stripped pavement layer in preparation for a thinner wearing layer placed over top.

Experience has shown that although CIR has many beneficial applications, it tends to be somewhat unpredictable when using current design and construction methods. The goal of this research is to improve the project success rate by improving the design process. Current design methodology ignores the time and temperature dependency of real world construction practices and does a poor job of predicting emulsion field performance needs. Current material specifications ignore the issue of timely return to traffic, as well as density and field stability.

1.2 Objectives

Issues addressed in mix design must relate to field performance both in construction and long term durability. Observation of construction practices has revealed a number of troubling departures from mix design. Areas of lack of control include:

- Gradation and its effects on achieving density and optimum emulsion content.
- Density and its effects on emulsion content, stability, cracking potential and permeability.
- Evaluation of proper emulsion addition rates, typically far less than predicted.
- Objective determination of proper time to return to traffic.

The goal of this research is to understand the drivers behind these departures from current mix design practices and to improve the predictive abilities of the mix design process. Several areas will be investigated including:

1. Does the treatment of a laboratory sample (and thereby the field produced pavement) effect stability?

2. How does RAP compactability respond to temperature?
3. How does RAP compactability respond to gradation?
4. How does increased density affect allowable moisture (all forms)?
5. How do changes in gradation affect allowable moisture (all forms)?
6. What are the drivers of emulsion break and how can they be managed?
7. Can emulsions be differentiated on the basis of constructability and release to traffic parameters?

1.3 Scope

Question 1: Does the treatment of a laboratory sample (and thereby the field produced pavement) effect stability? This question was broken into two parts.

- Does the sample production temperature affect stability? One RAP source, two emulsions and four temperatures were used for comparison. All were mixed with 6% total moisture, 2.0% emulsion and 1.5% lime. They were then cured for 2 hours prior to compaction. All were compacted to 50 gyrations and held for 8 hours prior to conducting the Marshall stability test. These protocols were developed from the handling of field produced mixes documented in earlier research for UDOT.
- Does post compaction curing time affect stability? One RAP source, two emulsions and one temperature were used for comparison. All were mixed with 6% total moisture, 2.0% emulsion and 1.5% lime. They were then cured for 2 hours prior to compaction. All were compacted to 50 gyrations. The samples were then held for 2, 4, 6, 8 and 10 hours.

Questions 2 and 3 were addressed in the same data set: How does RAP compactability respond to temperature? Also, how does RAP compactability respond to gradation?

These two questions were addressed using the following experimental design.

RAP millings were selected from two of the major climate areas in the State of Utah.

Specifically: SR-32 Jordanelle, US 40 Strawberry and Southern Parkway Saint George. The range of climates is Dry/No Freeze and Wet/Freeze. This selection produces RAP qualities from very hard to semi-soft. Each RAP was screened and medium and coarse mix gradations were produced. Each mix gradation heated to the test temperature and combined with 2% water by weight. No emulsion was added. Compaction was done in the Superpave Gyratory Compactor (SCG) at test temperature \pm 1°F to 50 gyrations. Three replicates were performed of each of the

four mixes. Test temperatures were as follows: 60, 80, 100, 120, 140°F. Compaction curves were recorded. Compaction densities were calculated and plotted. Average densities were determined and plotted for each temperature, gradation and RAP source. Trends were observed.

Questions 4 and 5 were handled from a single data set. How does increased density affect allowable moisture (all forms)? And, how do changes in gradation affect allowable moisture (all forms)?

The experimental design used the Southern Parkway RAP source. A medium and coarse mix was blended from screened material. Three replicates of each mix were tested. Water was added incrementally to each mix using a Modified Proctor procedure to determine optimum moisture.

Questions 6 and 7 involve all we have learned from questions 1 thru 5. What are the drivers of emulsion break? How can they be managed? Can emulsions be differentiated on the basis of constructability and release to traffic parameters? A number of factors were considered to develop a test which will answer this question. The specifics of these factors will be discussed later but the final experimental design requires mixing minus #30 screen material with water, lime and emulsion, consolidating the mix sample by vibration, holding the samples at the test temperature for periods of 10, 20, 30, 45 minutes and 1, 2, 3, 4, 6 and 24 hours. At each time increment, a custom designed cone penetration test is performed and the peak penetration force is recorded. Maximum % strength increase limits are placed on the sample during the first 30 minutes and Minimum % strength increase limits are set after 3 hours.

1.4 Outline of Report

- Introduction
- Research Methods
- Data Collection
- Data Evaluation (or Analysis)
- Conclusions
- Recommendations and Implementation

2.0 RESEARCH METHODS

2.1 Overview

Traditionally, the design of a CIR mix is done by sampling, crushing and grading the RAP into two gradations. Compacted samples are then prepared at a single temperature in the Superpave Gyratory Compactor (SGC) using varying emulsion contents. These samples are then tested for Marshall Stability and Flow (rutting), Tensile Strength Ratio (moisture susceptibility), Indirect Tensile (cold temperature cracking) and Raveling (loss of material through abrasion). An additional test was added around 2007 in the form of a Disk Shaped Tension Test (DCT) to measure intermediate temperature cracking potential. This system of tests should result in a balanced mix design with four of the five major distress mechanisms optimized. The difficulty is that the mix, as designed, is impossible to build. For several reasons which became apparent in prior UDOT research, emulsion quantity was regularly reduced in the field. The cause of these reductions will be shown in this research to be related to temperature sensitivity of the RAP source causing variations in gradation and compaction. These sensitivities in turn affect available void space and available specific surface.

Traditional CIR mix design has also concerned itself with ultimate mix performance while giving no thought to short term strength development. This research will provide some insight to the short and intermediate term demulsification properties of the emulsion. A new test procedure is developed to discriminate between emulsions exhibiting behavior conducive to CIR and those failing to produce acceptable characteristics.

Five tools were used to develop results in this investigation. They are as follows:

- Modified Proctor Compaction AASHTO T 180 method C.
- Modified Marshall Stability AASHTO T 245.
- Superpave Gyratory Compactor (SGC) AASHTO T 312
- Semi-Circular Bending – Modified Method
- Forced Cone Penetration – New Method

2.2 Background for Modified Proctor Compaction AASHTO T 180

Two issues were investigated with this procedure. Both were pursued on compacted, dried samples. This procedure is used to determine the optimum moisture content required to produce maximum density in a coarse granular material. A 56 blows with a 10 pound hammer dropped 1.5 feet, is imposed on a material sample contained in a 6 inch diameter mold with a volume of 0.075 ft^3 (Figure 1). The sample is built up in 5 equal lifts. When compaction is complete, the top is leveled and a mass recorded. This process is repeated across a sweep of moisture content (2, 4, 6, 8, and 10%) and the mass is plotted against moisture. The peak mass vs moisture is where the moisture lubricates the compaction without pushing the particles and is termed the Proctor Density. In an unbound material, the particles are discreet and temperature plays no part. Since RAP is thermosetting, all tests were done at 100°F.



Figure 1. Proctor Hammer

A single RAP source was used for this experiment. The SR-32 project RAP was screened and two gradations (Medium and Coarse) were tested. Table 1 tabulates the gradations.

Screen	Medium Gradation	Coarse Gradation
Size	Percent Passing	Percent Passing
½"	86	73
¾"	77	62
#4	52	38
#8	33	20
#16	16	8
#30	7	3
Minus #30	0	0

Table 1: RAP Gradations for Proctor Testing

2.3 Background for Marshall Stability AASHTO T 245

AASHTO T 245 is a method of determining the rut susceptibility in a Hot Mix Asphalt. Both saturated and unsaturated flow (distance to maximum load) and stability (maximum load) are determined. The test was originally set up to use the 4" Marshall pill but has been modified to accept the 6" SGC puck. Marshall Stability is determined by:

$$St = \frac{2 * P}{3.14 * D * t} \quad (2.3.1)$$

Where:

S_t = tensile strength (psi)

P = maximum load (lb)

D = specimen diameter (in)

T = specimen thickness (in)

A Marshall Stability machine is shown in Figure 2



Figure 2: Marshall Stability Machine

Two issues were investigated with stability. The first was how does stability vary with temperature? The second, how does stability vary with time? As to the first question, the experimental plan was as shown in Table 2

Conditions of Test		Temperatures (deg F)			
		60	80	100	120
Aggregate and lime Weight (g)	3500	X	X	X	X
Water Weight (g)	140.8	X	X	X	X
Emulsion Weight (g)	58.5	X	X	X	X
Gradation	Medium	X	X	X	X
Gyrations in SGC	50	X	X	X	X
Solventless Engineered Emulsion		3 *	3	3	3
CSS-1 Emulsion		3	3	3	3

* number of replicates

Table 2: Stability vs Temperature Experimental Plan

The RAP was from I-84 at Henefer. RAP samples were conditioned dry for at least 4 hours to bring the materials to the mixing temperature. Lime, water and emulsion, in that order, were added and the resulting mix was sealed in a plastic bag where experimental test temperature was maintained for 2 hours prior to compaction. The SGC compactor molds and plates were brought to test temperature and the samples were densified. The samples were placed back in the environmental chamber where temperatures were maintained for 8 hours. The stability test was then performed on the cured, unsaturated sample.

Table 3 is the experimental plan to answer the second question.

Experimental Conditions		Time (hours)				
		2	4	6	8	10
Aggregate and lime Weight (g)	3250	X	X	X	X	X
Water Weight (g)	140.8	X	X	X	X	X
Emulsion Weight (g)	58.5	X	X	X	X	X
Gradation	medium	X	X	X	X	X
Gyrations in SGC	50	X	X	X	X	X
Temperature (Degrees F)	80	X	X	X	X	X
Solventless Engineered Emulsion		3*	3	3	3	3
CSS1 Emulsion		3	3	3	3	3

* number of replicates

Table 3: Stability vs Time, Experimental Plan

The RAP was from I-84 at Henefer. RAP samples were conditioned dry for at least 4 hours to bring the materials to the mixing temperature. Lime, water and emulsion, in that order, were added and the resulting mix was bagged and 80°F was maintained for the 2 hours prior to compaction. The SGC compactor molds and plates were brought to test temperature and the samples were densified. The samples were placed back in the environmental chamber where 80°F was maintained for the experimental duration. The stability test was then performed on the cured, unsaturated sample.

2.4 Background of Superpave Gyratory Compactor AASHTO T 312

The SGC is the central player in Superpave volumetric design. With this device, accurate and repeatable compaction energy can be imparted to a sample. Temperature can be controlled reasonably well over a few minutes due to the mass of the molds and sample. Two to four degrees temperature drop can be expected if sample materials are properly heated and compacted with reasonable dispatch. The high compactive forces render the test relatively insensitive to this magnitude of temperature variation. The device both applies pressure and kneading to simulate the compaction roller in the field. The height of the sample is recorded at each gyration creating a record of the compaction curve. The volume of the sample can therefore be determined at any point in the procedure.

Compaction densities can be then calculated using the following scheme:

$$\pi * r^2 * h = V \quad (2.4.1)$$

Where r and h are in cm

$$G_{mm} * V_{cs} = W_{mm} \quad (2.4.2)$$

Where G_{mm} is the maximum specific gravity of the mix (Rice)

V_{cs} is the Volume of the compacted sample

W_{mm} is the maximum weight of the sample if no air voids are present

$$(1 - W_{sdry} / W_{mm}) * 100 = \% \text{ Air} \quad (2.4.3)$$

Where W_{sdry} is the weight of the dry aggregate

$$100 - \% \text{ Air} = \text{Density of the sample} \quad (2.4.4)$$

A photo of a typical SGC is shown in Figure 3



Figure 3: Superpave Gyratory Compactor

The questions to be answered with this device are whether the compaction of a RAP is sensitive to temperature and if so, is the temperature sensitivity unique to the RAP source. To answer these questions, the following experimental procedure was executed:

RAP millings were selected from two of the major climate areas in the State of Utah. Specifically: SR-32 Jordanelle, US 40 Strawberry and Southern Parkway Saint George. The range of climates is Dry/No Freeze and Wet/Freeze. This selection produces RAP qualities from very hard to semi-soft.

A medium and fine gradation, as described in section 2.2 Table 1, were prepared for each RAP source.

Each mix gradation heated to the test temperature and combined with 2% water by weight. No emulsion was added. Compaction was accomplished in the SGC at test temperature $\pm 1^{\circ}\text{F}$ to 50 gyrations. Three replicates were performed of each of the four mixes. Test temperatures were as follows: 60, 80, 100, 120, 140°F

2.5 Background of Semi-Circular Bending

Great interest has been given to fracture mechanics in asphalt pavements in recent years. Many tests have been developed in an attempt to understand low and intermediate temperature cracking. One test showing great promise due to its simplicity is the Semi-Circular Bending test. The configuration used in this study is shown in Figure 4.

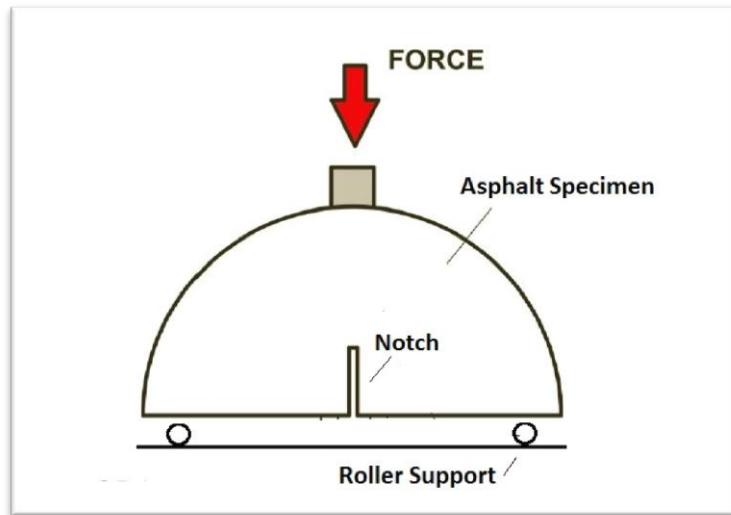


Figure 4: Semi-Circular Bending Configuration

The center crack is loaded in three point bending where load and displacement are plotted. The tests were run at 2 inches per minute and 80°F. The sample was 153mm in diameter with a 9mm tall notch. Two properties are of interest in this test. The first is Fracture

Toughness. The Second is Fracture Energy. (Biligiri et.al, 2012) Fracture toughness is a function of the maximum stress and the cross section of the beam. The formulation is as follows:

$$\sigma_{\max} = \frac{4.263 \times f_{\max}}{D \times t} \text{ N/mm}^2 \quad (2.5.1)$$

Where:

D = Diameter of sample (mm)

T = Thickness of Sample (mm)

Fmax = Maximum Force in Newtons

For notched SCB specimens with s/r = 0.8, fracture toughness is as follows

$$K_{IC} = \sigma_{\max} \sqrt{\pi a} \times f\left(\frac{a}{w}\right) \text{ N/mm}^{\frac{3}{2}} \quad (2.5.2)$$

Where:

W = Height (mm)

a = Notch depth (mm)

σ_{\max} = Stress at failure calculated in equation 2.5.1

$f\left(\frac{a}{w}\right)$ = Geometric Factor

$$= 4.782 - [1.219(a/r)] + [0.603e^{[7.045(a/r)]}]$$

Where:

s = half of the spacing between rollers = 60mm

r = radius of the specimen = 75mm

The fracture energy of the material is the work done on the material to increase fractured surface within the area of the ligament.

Work is the area under the load/deformation curve obtained by summing up the height of the curve incrementally from zero to the maximum force.

Area of the ligament is the radius of the sample minus the notch height times the sample thickness.

Given the previous discussion, and removing the constants, four parameters rise to the surface for investigation. Maximum force, displacement at fracture, slope of the force/displacement curve and the area under the force/strain curve tell the basic story of fracture toughness and fracture energy. Two of these are measured and two calculated. The two measurements will be taken in the following experiment.

Experimental design:

- Field mix gradation

- One RAP source, Geneva Rock Products, Gradation: Field
- Five emulsions: PassR, RS.EE, E.EE, CQS 1, CSS 1
- Replicates = 2 Pucks, 4 tests
- Cure Time: 24, 48, 72 hours.
- Temperature = 80°F.
- Sample preparation:
 - Bring all materials to temperature for 4 hours
 - Mix 2950g of RAP with 1% lime, 3% water and 2.5% emulsion
 - Condition sample at test temperature for 2 hours.
 - Compact sample to constant height/density ~ 15% void in heated compactor molds
 - Cure sample for 12 hours
 - Cut sample in half and notch
 - Condition sample for 2 hours to equalize temperature
 - Break the sample and record the data.
- Test Speed = 2 inches/minute
- Test Apparatus: Humbolt HM-3000 with a with a 4 k newton load cell.
Recording rate: 500/sec



Figure 5: Humbolt HM-3000 w 4kn load cell

2.6. Background for Forced Cone Penetration

Engineered emulsion performance tests are typically used to evaluate final mix properties after demulsification is complete. These tests are valuable but fail to characterize construction properties which are left to the field for adjustment. A method to understand the cure rate of an emulsion is needed to identify both construction and long term behavior. Emulsions break in a number of ways including evaporation, water loss by absorption into adjacent surfaces, chemical neutralization, exposure to vibratory energy and as many others a clever engineers can imagine. In developing a test which adequately investigates construction properties, a number of issues were considered.

Size of sample: It was observed that to look at gains in shear resistance (increasing viscosity) over time, a number of samples would have to be created at time zero and tested over time increments of interest. This would involve creating as many as ten uniform samples. Creating a single batch and dividing it into small samples seemed a logical way to proceed. Looking at the disposable Dynamic Cone Penetrometer head as a potential test apparatus and the minus #30 screen RAP as the mortar producing portion of the sample, a 2 oz. vinyl cup produced by Silo was determined to be of adequate diameter and depth to contain an 80g test sample. A uniform sample could be produced rapidly from a larger batch without producing aggregate or moisture segregation. If the fine particles were supersaturated with fluids, the cup would be sufficiently larger than the test head so that viscosity could be measured without interference from friction at the zero-slip surfaces.

Size of aggregate: The goal of this test procedure is to measure the relative viscosity gain in an emulsion/mortar system in a specific temperature range. The mortar fraction is generally considered to consist of the aggregates passing the break screen. (in this case #8) Using particles up to this size greatly increases the required sample size with no apparent change in test outcome. Reducing the particle size to those passing the #30 screen allows much better control over water content and allows the sample container to be 2.5 inches in diameter and 1.25 inches deep.

Moisture: Experiments were run to determine how sensitive the emulsion cure time was to moisture content. Minus #8 screen RAP together with lime was mixed with emulsion only (no added water). Lime and emulsion were proportioned for the full mix at 1% and 2.5% respectively. Break was immediate and no cohesion was observed. Conclusion: Absorption against dry aggregates and other surfaces causes rapid demulsification. A moisture sweep was then conducted to determine the requirement to prevent immediate emulsion break. This sweep

consisted of adding moisture as a percent of lime. A 1:1, 2:1 and 3:1 water to lime ratio was used. Rapid break was observed in all cases but aggregate coating improved along with adhesion. Conclusion: Since the lime was applied to the small aggregates at the rate prescribed for the full mix and the water was applied as a ratio to the lime, all of the water from the lime slurry was taken up by the minus #8 portion of the mix. In the case of the medium mix, this portion is only 30% of the total aggregate. Further addition of water up to liquefaction was required to prevent flash demulsification. Moisture in excess of supersaturation was shown to reduce initial viscosity and to slow the increase of viscosity in both the near and long term.

Moisture content as a % of fines/total mix: Two mix design gradations were provided in the standard Road Science mix design. They are as presented in Table 4.

Screen	Medium Gradation		Coarse Gradation		
	Size	% Passing	% Retained	% Passing	% Retained
½"		86	14	73	27
3/8"		77	9	62	11
#4		52	25	38	24
#8		33	19	20	18
#16		16	17	8	12
#30		7	9	3	5
Minus #30		0	7	0	3

Table 4: RAP Gradations

The initial questions on moisture come from work done in the Proctor test using the full mix. What is the water content at maximum Proctor Density? How much of this water is contained in the minus #8 material? How much of this water is contained on the minus #30 material? These questions are relevant to the requirement that the minus #30 material must be supersaturated to prevent flash emulsion break.

Proctor results on the medium gradation mix show that density increases until free water is observed. Adding additional water has no effect on density, it simply runs out of the mix. This value was 8% moisture for the medium and 6% for the coarse gradation. The top of the curve was very flat in both cases making it difficult to be more precise than 1%. The water at optimum for the mix has been calculated as a percent of the minus #8 and minus #30 fines. This is to determine if the fines are super-saturated at optimum moisture. Results are shown in Table 5.

Gradation					Total Fines (g) 100g sample	Water (g) Full Mix Proctor	Proctor Water % of Fines	
	% Passing	Lime	#8	#30			#8	#30
Medium	33	7	1.5	34.5	8.5	8	23.2	94.1
Coarse	20	3	1.5	21.5	4.5	6	27.9	133.3

Table 5: Optimum Moisture (Proctor Test)

Moisture sweeps were run on the full gradation, the minus #8 and the minus #30 fines to determine supersaturation condition. This condition is defined as the point where sufficient moisture is present in the aggregate to produce liquefaction under a vibratory input. The vibration in this experiment was produced by a concrete consolidation vibrator. Supersaturation is required to prevent flash demulsification when emulsion is added. For the medium gradation, supersaturation was reached at 5% water while the coarse reached this point at 2.6%. Results for the minus #8 and minus #30 are in Table 6. These results demonstrate that 95 to 100 percent of the moisture required to prevent flash demulsification is contained on the minus #8 screen and that when using this technique to determine optimum water content, the percent passing both #8 and #30 screens must be measured.

Water to liquifaction of fines (g) 100 g sample		% minus # 30 in the #8 fines	Water in the #8 on the minus #30	Water in the #8 on the minus #30	% water in the mix based on the liquifaction of the minus #8 fines	% water in the mix based on the liquifaction of the minus #30 fines
#8	#30		grams	Percent		
15.0	36	21	7.6	50.9	5.0	2.5
15.0	36	15	5.4	36.0	3.0	1.1

Table 6: Water to Reach Supersaturation.

Upper line = Med, Lower line = Coarse gradation

Load Cell: All test results fall in the one to ten pound range. This is less than 5% of a 200 pound load cell. It would be much better if a 50 pound load cell was available. These cells are available from “Interface” and “Celtron” at a reasonable price.

Head Configuration: Several head configurations were tried. 3/16 flat, 5/16 flat and 3/4 inch-90 degree cone were investigated. Only the cone produced measured forces sufficiently high to demonstrate variation in viscosity. The challenge with this configuration is that the force increases with the increasing diameter of the cone and the increasing contact area. Care must be taken to place the tip of the cone on the surface of the slurry and penetrate the sample to the same depth each time.

Vibration/Consolidation: One of the inputs to the CIR mat in the field is the vibration from the knock down roller. This input has been discussed as a possible energy input in the demulsification process. In the field, the vibration intensity and amplitude is variable based on the compactive needs of the mat. It is necessary to standardize this input in the lab. A concrete consolidation vibrator was chosen to provide the necessary input and a duration of 15 seconds was similarly chosen. The effect of this operation at the fluid levels developed above is to liquefy the mortar and fully disperse the particles in fluid. All consolidation from this point is either settlement or demulsification.

Temperature: While working with full pucks, it was demonstrated that temperature dominates demulsification. In the field, the most common processing temperature was between 80 and 100°F. 100°F is therefore the critical temperature for constructability. The emulsion must not break in the windrow at this temperature. For this reason, 100°F was chosen as the test temperature for emulsion performance qualification.

Speed: A sweep of head speed was conducted from 0.1 inches per minute to one inch per minute. The best resolution at the most reasonable test duration was 0.5 inches per minute.

Time: Time is a critical factor in both constructability and release to traffic. The emulsion break must delay for enough time to pass through the paver but must develop viscosity in time to release to traffic. Twenty minutes is sufficient time to pass the paver and three hours is sufficient to release to traffic. The emulsion must therefore show minimal viscosity gain over the first twenty minutes but must show significant gain in the next three hours.

Test Procedure: With deference to the considerations presented above, a Cone Penetration Test is proposed for Qualifying a Engineered Solventless Cold In Place Recycling Emulsions (ESCIRE). The test consists of driving a cone into a sample of emulsion/RAP mortar and measuring the resistance at time increments between 10 minutes and 24 hours. See appendix 1 for the complete test procedure.

Experimental Plan:

- One RAP source, Southern Parkway, minus #30 sieved fraction.
- Three Emulsions, RS-EE from SR-32, Erg-EE and Erg-CSS1h provided to the project for research purposes.
- Temperature 100°F.
- Penetration rate 0.5 inches per minute.

The cone apparatus and the sample are shown in Figure 6. The test equipment is shown in Figure 7



Figure 6: Cone Apparatus and Sample of RAP mortar.



Figure 7: Cone Penetration Test Equipment

3.0 DATA COLLECTION

3.1 Overview

Pavement performance is a balancing act between competing mix properties. An optimization must be developed between rutting resistance and cracking resistance. CIR has additional challenges including, a need for short term stability, a lack of gradation control, temperature sensitive amorphous particles, a need for space to store fluids during construction and a need for long term fracture energy control. The following tests are proposed to give insight into these issues:

- Modified Proctor Compaction AASHTO T 180 method C. (Fluid capacity)
- Modified Marshall Stability AASHTO T 245. (Short and long term stability)
- Superpave Gyratory Compactor (SGC) AASHTO T 312 (Compactability)
- Semi-circular Bending (Long term fracture energy)
- Forced Cone Penetration (Short term stability behavior)

Data from these tests are presented below.

3.2 Modified Proctor Compaction AASHTO T 180 method C.

To determine the optimum compaction moisture at 100°F (a common mat temperature in the field), SR-32 RAP was screened. Coarse and Medium gradations were prepared and compacted at the target temperature. The weight of a unit volume of compacted material was recorded at each of five moisture contents.

The peak of the graph is the optimum moisture content.

Figure 8 illustrates the recorded data for the medium gradation and Figure 9 for the coarse gradation.

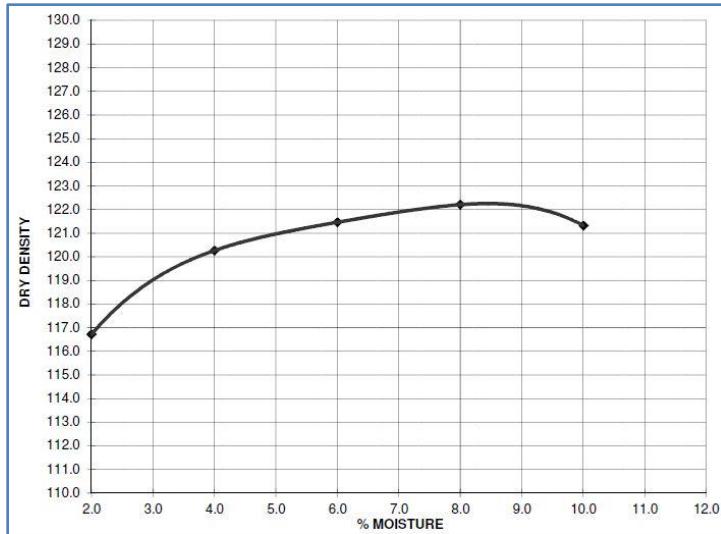


Figure 8: Proctor Graph: Optimum Moisture Medium Gradation

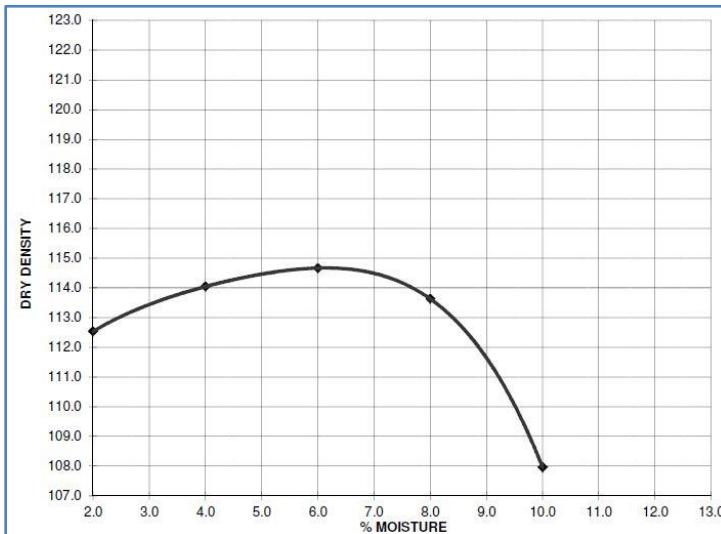


Figure 9: Proctor Graph: Optimum moisture Coarse Gradation

At 6% moisture in the coarse and 8% in the medium gradations, free water began to appear in the samples. Beyond this point, water was draining from the sample during compaction. This is not an unusual result when testing non plastic, open- graded, unbound materials.

3.3 Modified Marshall Stability AASHTO T 245.

The Marshall Stability procedure is a basic test for stiffness characterization of asphalt mixes. Early stages of the research were focused on two mix characteristics using this procedure. They include;

- How does the sample stability change with change in temperature?
- How does the stability of the sample change with changes in curing time?

The goal of the investigation of the time and temperature variations in stability was to determine if certain temperature ranges or curing times could be used to differentiate between different emulsions and their rate of early strength gain during construction.

3.4 Test Procedures

The test procedures for both of these phases are very similar, with changes primarily being made to either the curing time used or the curing and testing temperature used. Specific parameters include:

Objective 1: Curing Temperature sensitivity of CIR Emulsion

Road Science Emulsion & CSS-1

Mix and cure @ 60, 80, 100, 120° F

Test 8 hour stability using Marshall Stability

Objective 2: Curing Time sensitivity of CIR Emulsion

Road Science Emulsion & CSS-1

Mix and cure @ 80° F

Test 4, 6 and 10 hour stability using Marshall Stability

3.4.1 CURING TEMPERATURE SENSITIVITY

RAP collected from the I- 84 project was used in the making of CIR pucks to compare the performance of different emulsions mixed, compacted and cured at different temperatures. Characteristics of the samples included:

1. Lime was already on RAP

2. 3250 grams of RAP were used in each sample.
3. 140.8 grams of water were used in each sample.
4. 58.5 grams of emulsion were used in each sample.
5. Pucks were compacted to 50 gyrations.
6. 3 -Road Science Reflex emulsion samples and 3-CSS-1 emulsion samples were made at 60, 80, 100 and 120° F each.
7. 3 –PMCIR, CQS and PASS-R samples were made at 60° F and 80° F each.

The RAP was quartered and split down to the 3250 gram samples from the buckets taken at the I-84 Henefer to Echo project. It was calculated that the 3250 grams of RAP with water and emulsion added would result in approximately 90 mm pucks. The bags of RAP were conditioned to the temperature used at mixing, compaction and curing of the pucks for at least 4 hours.

The RAP was combined with water and emulsion and mixed until there was uniform coating of the RAP. The quantities of RAP, water and emulsion were measured and recorded.

Note: The Road Science emulsion would combine with the RAP and leave very little residue in the mixing bowl. The CSS-1 emulsion left considerably more residue in the mixing bowl especially at lower temperatures.

The mixed samples were bagged in Ziploc bags and kept at the test temperature for 2 hours before compaction. The samples were compacted at 50 gyrations.

Note: The height of the samples at 50 gyrations changed with the change in temperatures from 84.1 mm average ht. at 120° F to 95.5 mm average at 60° F. The amount of water forced from the mix at compaction changed accordingly with little to no water in gyro at 60 degrees.

The Road Science pucks had less moisture between the puck and the plates of the gyratory compactor so the plates were considerably more difficult to remove.

The pucks were weighed after compaction and kept at the test temperature for 8 hours. They were weighed again prior to stability tests. Stability tests were performed and the results recorded.

Note: The pucks done at 120° F had lower stability numbers, however the pucks showed little cracking or crumbling as a result of the test. The cooler temperatures increased the amount of crumbling and cracking especially on the CSS-1. The Stability test results were highest on the

80° F pucks however the CSS-1 pucks at 80° F and both sets of pucks at 60° F were cracking and breaking apart while removing the puck from the test mold.



Figure 10: Reflex puck after stability test at 120 degrees



Figure 11: Reflex Puck after Stability at 80 degrees



Figure 12: CSS-1 Puck after Stability at 80 Degrees



Figure 13: Reflex Puck after Stability at 60 degrees.

3.4.1.1 Curing Time Sensitivity

RAP collected from the I- 84 project was used in the making of CIR pucks to compare the performance of different emulsions at different amounts of time to test after compaction. As with the temperature sensitivity investigation, the characteristics of the samples included:

1. Lime was already on RAP
2. 3250 grams of RAP were used in each sample.

3. 140.8 grams of water were used in each sample.
4. 58.5 grams of emulsion were used in each sample.
5. Pucks were compacted to 50 gyrations for the 4 and 6 hour samples and 90 mm height for the 2 and 10 hour samples.
6. 3 -Road Science Reflex emulsion samples and 3-CSS-1 emulsion samples were made at 80 degrees and tested at 2, 4, 6, 8, 10 hours after compaction.

RAP, water and emulsion were measured out, recorded and mixed similar to Task 1 at 80° F. The samples were placed in Ziploc bags and stored at 80° F for 2 hours prior to compaction.

Note: The Reflex emulsion still left little residue in bowl after mixing while the CSS-1 emulsion left residue in the mixing bowl.



Figure 14: Road Science Reflex after mixing



Figure 15: CSS-1 after mixing

The samples were compacted 2 hours after mixing. The 4, 6, and 8 hour to stability test samples were done first. They were compacted to 50 gyrations and placed in an 80° F oven until Stability tests were performed. After these were complete it was decided that going forward the pucks would be made to a set height of 90 mm instead of at 50 gyrations to keep the voids similar rather than the compaction effort. The 2 and 10 hour to stability test pucks were made to 90 mm height which required 63-85 gyrations.

Note: There was some water forced out of the mix at compaction and the CSS-1 had more moisture under the plates making the plates easier to remove.

The weight of the pucks was recorded after compaction and prior to stability testing. Stability tests were performed at the set times after compaction. The 4 and 6 hour results were very similar to the 8 hour results from the pucks made with Task 1.

Note: The 2 hour pucks had higher stability test numbers than pucks tested at other times after compaction. I thought this may be due to the extra compaction effort used to bring them to 90 mm however the 10 hour pucks, which were made similar to the 2 hour pucks had test results very similar to the 4, 6 and 8 hour pucks made previously at 50 gyrations.

3.5 Superpave Gyratory Compactor (SGC) AASHTO T 312

The SGC was used to see whether RAP compaction is temperature sensitive and if so, is this sensitivity unique to each RAP source. Constant mass samples were compacted with 2% water at various temperatures to 50 gyrations. The results were graphed.

3.5.1 US-40 MEDIUM AND COARSE GRADATIONS

These graphs demonstrate the temperature sensitivity of the US 40 RAP. Each colored curve represents the compaction of the material at a specific temperature. This RAP comes from a high altitude, cold temperature area.

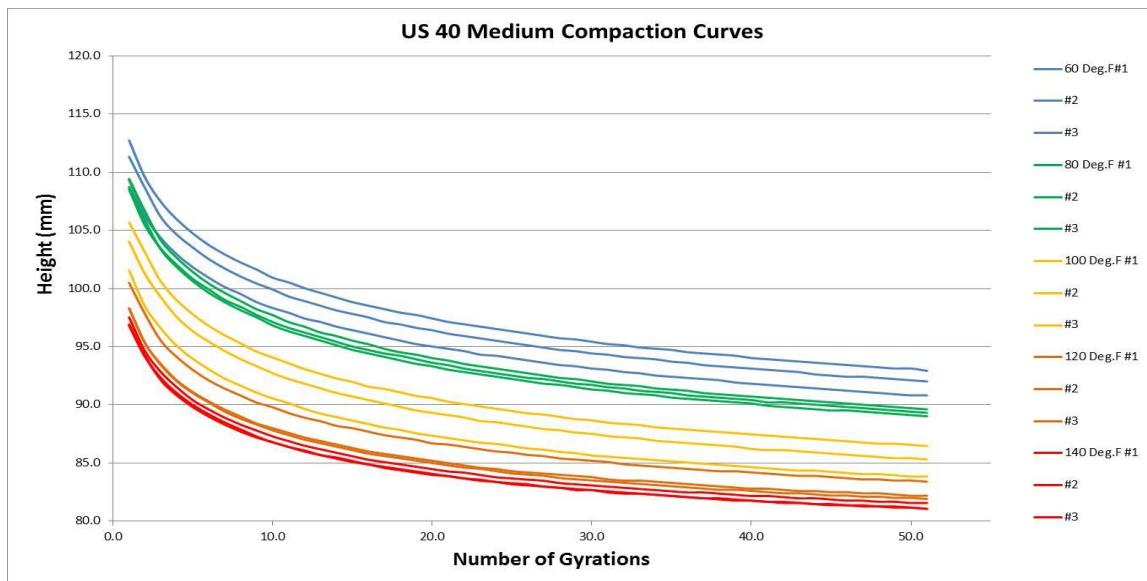


Figure 16: US-40 Medium Gradation Compaction Curves

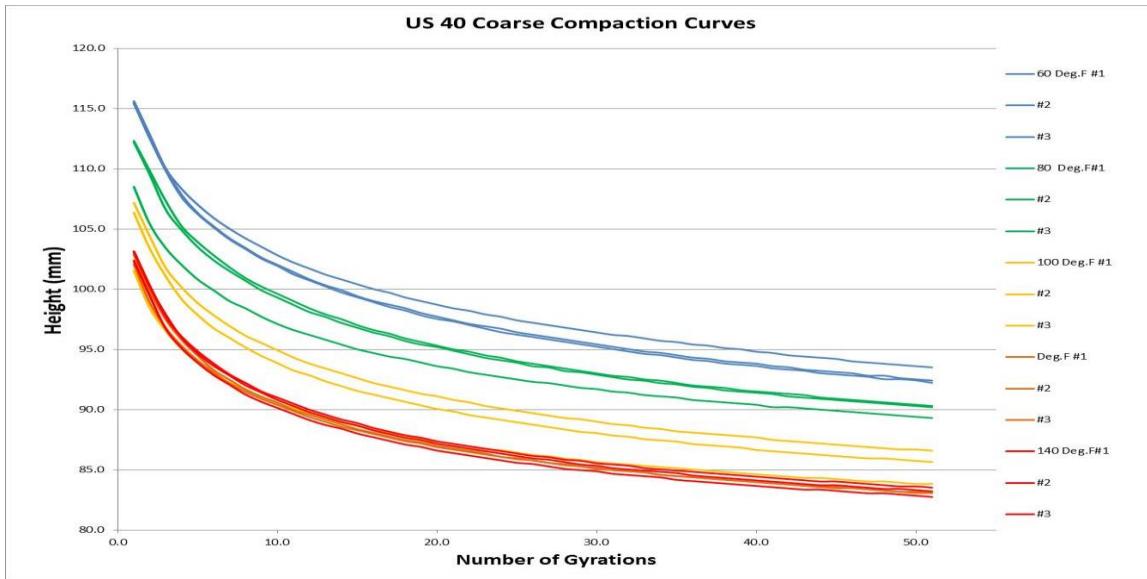


Figure 17: US-40 Coarse Gradation Compaction Curves

3.5.2 SR-32 MEDIUM AND COARSE GRADATIONS

These graphs demonstrate the temperature sensitivity of the SR-32 RAP. Each colored curve represents the compaction of the material at a specific temperature. This RAP comes from a high altitude, cold temperature area.

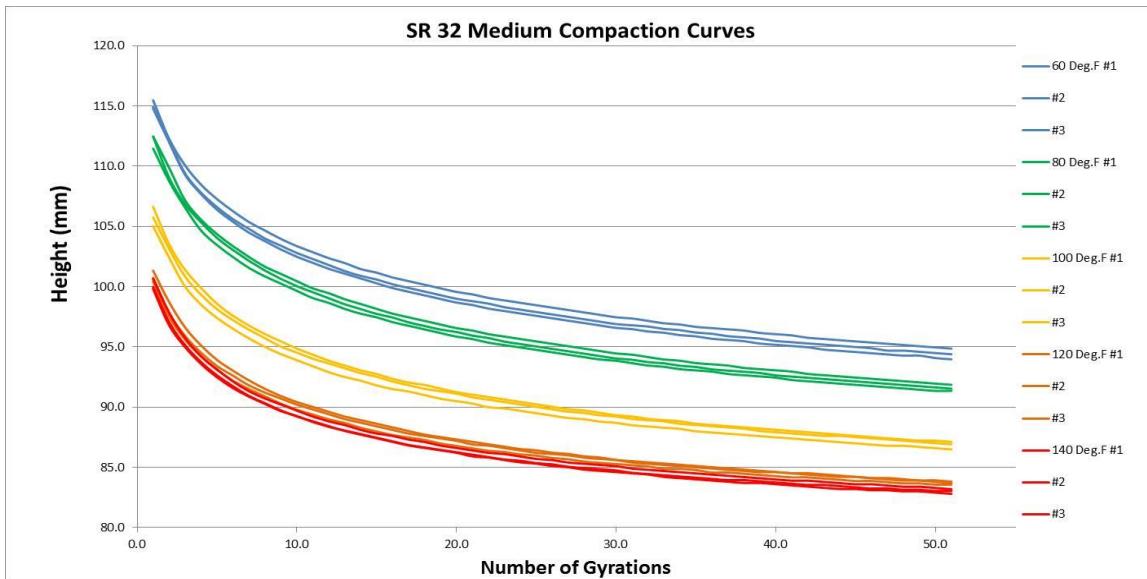


Figure 18: SR-32 Medium Gradation Compaction Curves

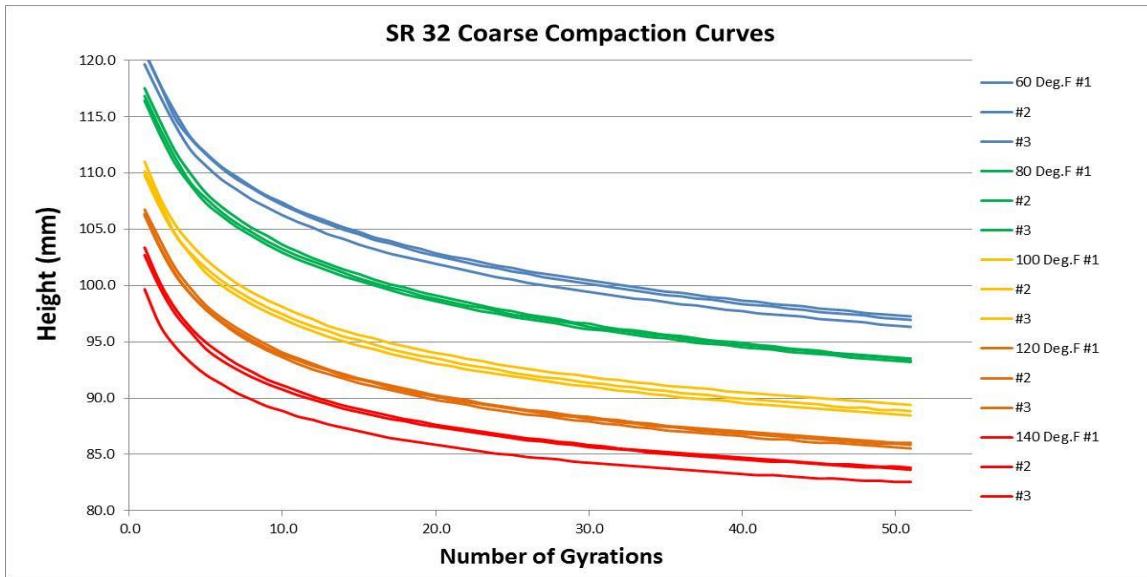


Figure 19: SR-32 Coarse Gradation Compaction Curves

3.5.3 SOUTHERN PARKWAY MEDIUM AND COARSE GRADATIONS

These graphs demonstrate the temperature sensitivity of the Southern Parkway RAP. Each colored curve represents the compaction of the material at a specific temperature. This RAP comes from low altitude, hot temperature area.

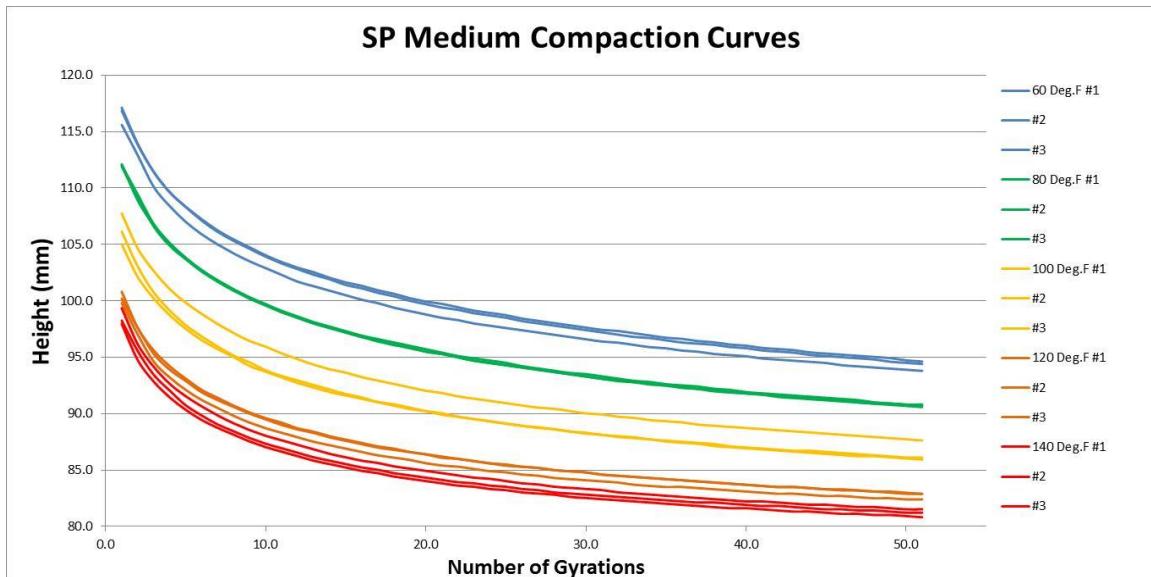


Figure 20: Southern Parkway Medium Gradation Compaction Curves

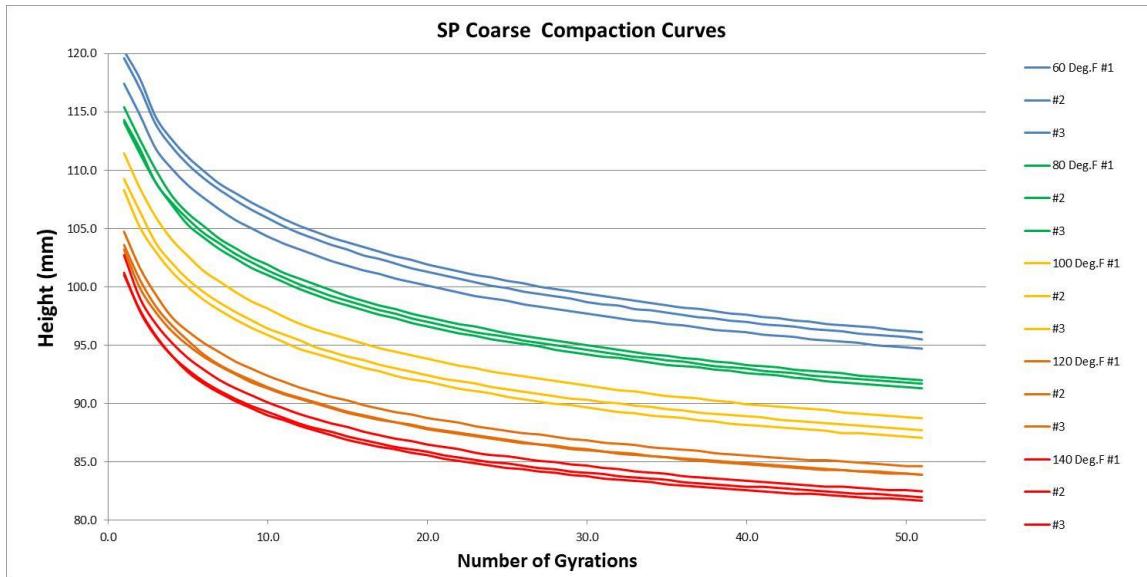


Figure 21: Southern Parkway Coarse Gradation Compaction Curves

3.6 Semi-Circular Bending

The 5 test results from the five emulsions are illustrated in Figure 22 through Figure 24.

3.6.1 TYPICAL RAW DATA

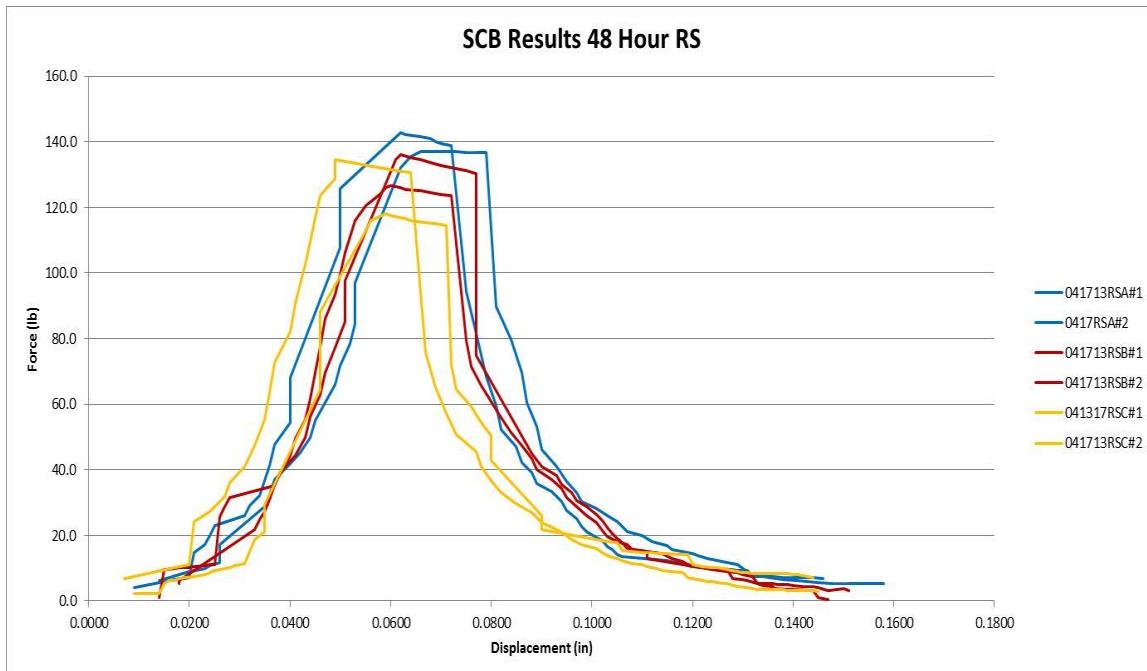


Figure 22: Typical SCB Results 3 pucks, 6 tests, 48 hour cure

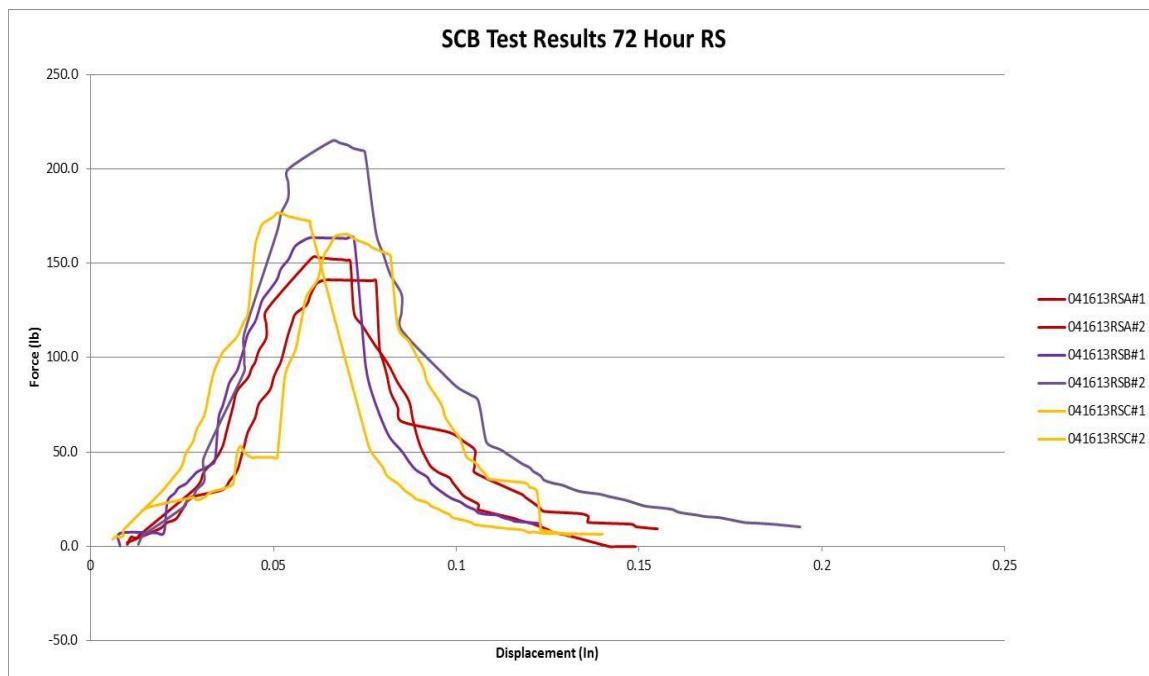


Figure 23: Typical SCB Results 3 pucks, 6 tests, 72 hour cure

3.6.2 PROCESSED DATA

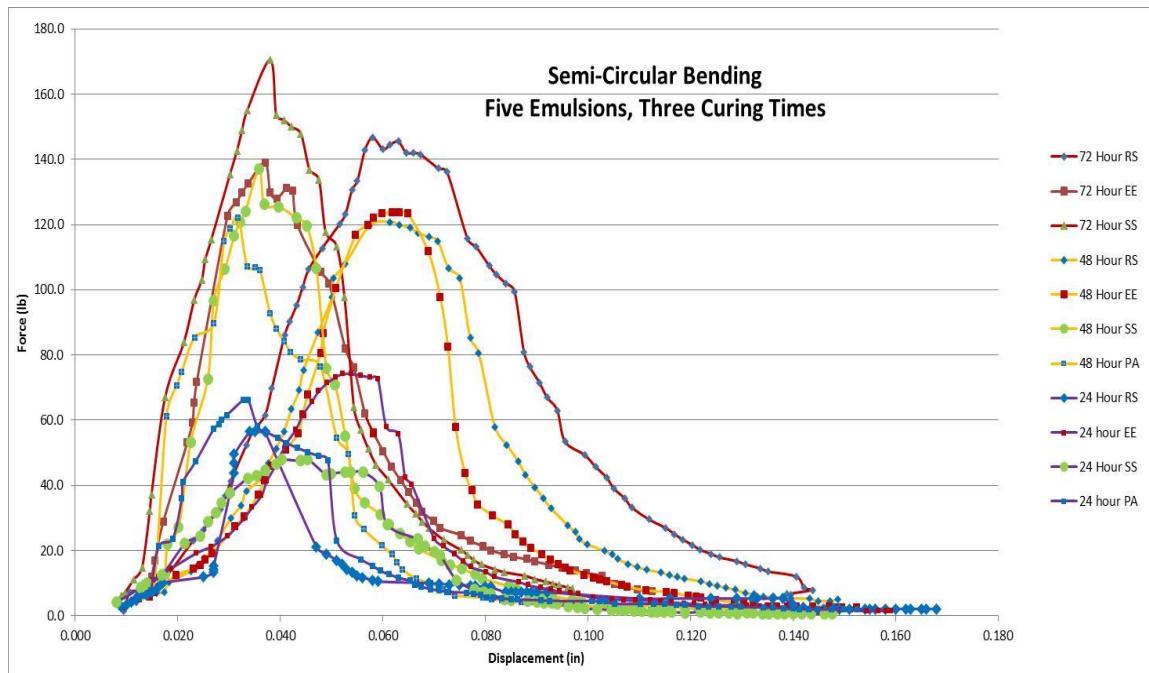


Figure 24: SCB results averaged from 6 tests for 5 emulsions and 3 curing times

The individual results have a fair amount of scatter leading to a need for better test controls. The foreleg of each curve is fairly well defined and a peak force value along with a displacement at that value can be determined directly from the data. The foreleg looks as if it will lend itself to regression fitting but a triangular model is a good start. The research will proceed on this basis with the recognition that improvements should be made.

3.7 Forced Cone Penetration

Test UT-01-14 was run on three emulsions. Two are ESCIRE and one is a slow set. Graphs of both maximum force vs time and % force vs time are presented in graphs 1 and 2

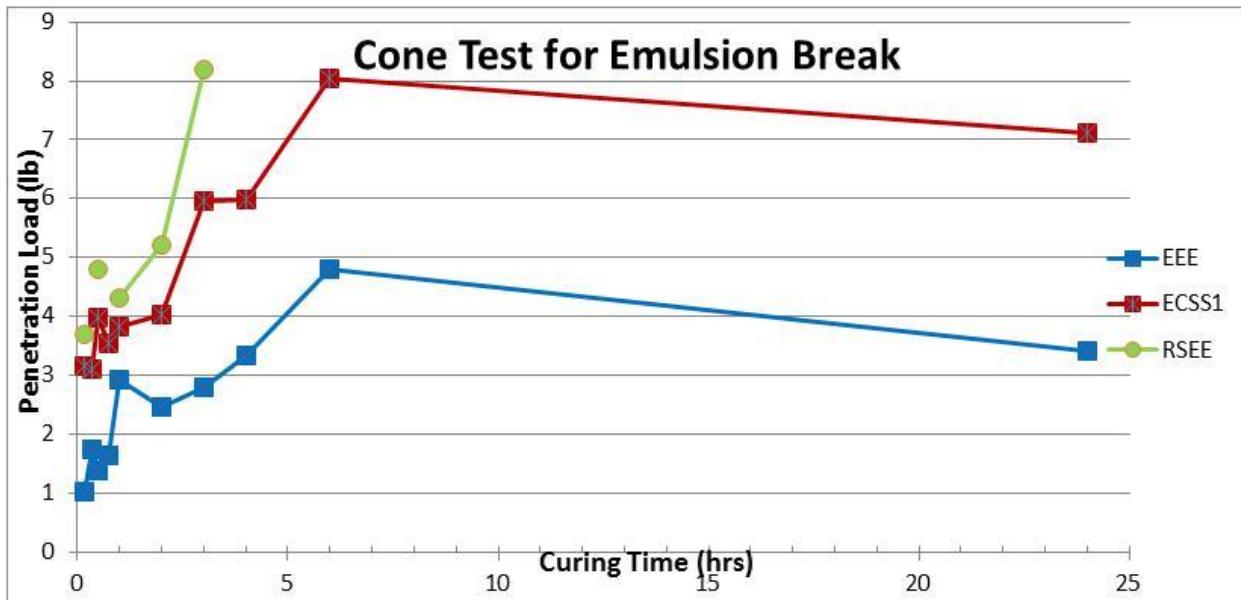


Figure 25: Penetration Load vs Time

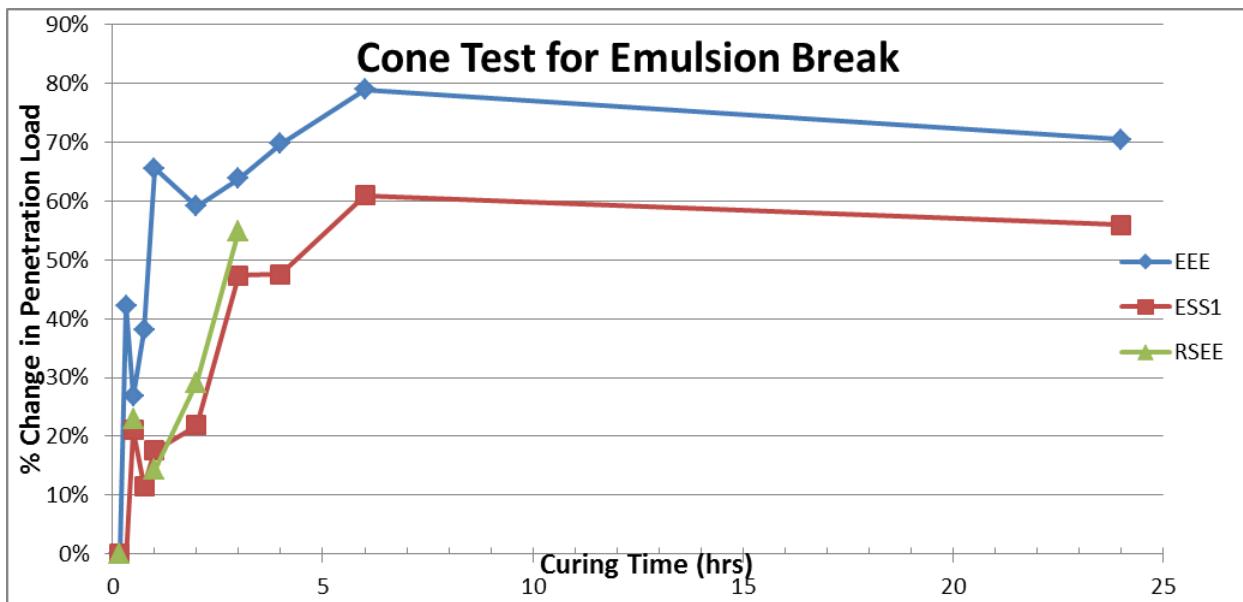


Figure 26: % Change in Penetration Load vs Time

Note that the percent change in the engineered emulsions is much greater than the slow set emulsion in the first four hours. This trend continues long after this initial set time. The initial stiffness as well as the near term stiffness shows no trend from one emulsion to another.

3.8 Summary

These experiments show that RAP particles are not discreet above 80° F but become more discreet above 120° F. Between these temperatures, RAP compaction behavior follows a predictable pattern. This pattern is offset vertically based on gradation in a predictable manner. They also demonstrate the temperature sensitivity in the development of stability. These observations are valuable when setting long term design values.

Fracture in CIR is a little understood property. It is unknown whether strong and resilient or weak and brittle is the correct attribute. Measuring this property is a first step in understanding what is desired in a base isolation interlayer.

Emulsion demulsification rate in the first three hours is imperative to control mixing, paving, compaction and return to traffic. The proposed test shows merit in understanding these properties.

4.0 DATA EVALUATION

4.1 Overview

Introduce the chapter contents. The Data Evaluation chapter includes information on how and why the data was evaluated. Statistical methods employed should be listed and their use justified. Extrapolated and evaluated data should be included on tables or charts or graphs that simplify and help understand trends or other information gathered.

4.2 Data Evaluation Modified Proctor Compaction. AASHTO T 180 method C.

Density in unbound materials is a function of compactive effort, moisture content and the surface area of the material. The more fines in a gradation, the more moisture it can hold before the voids are filled and the particles are pushed apart resulting in lower density. When a gradation has a high percentage of large particles, it can hold very little moisture before density is reduced and free moisture begins to drain from the sample. The data indicates that when the percent passing the #8 screen falls by 40%, the optimum moisture falls by 50%. This leads to a table of optimum moisture based on the percent of the sample passing the #8 screen.

	% passing #8	Optimum Moisture (%)
Fine Gradation	50	13.5
Medium Gradation	33	8
Coarse Gradation	20	6
Extra Coarse Gradation	13	3.6

Figure 27: Optimum Moisture Based on % Passing #8 Screen

4.3 Data Evaluation Modified Marshall Stability AASHTO T 245

4.3.1 SAMPLE STABILITY WITH TEMPERATURE

Observation of the results of the stability versus test temperature testing exhibited the typical decrease in the stability values with increases in temperature seen in other portions of the research. Pucks for the Road Science and CSS-1 emulsions were compacted at 50 gyrations,

where the remaining emulsions were compacted to a target height of 90 mm. Comparison of the results should be done in two categories. When compacting to a target density (height), the stability results show the typical downward trend of stability values with increased temperature (Figure 28). For the emulsions compacted with consistent compactive effort, the downward trend is present from 80° F and above, however the 60° F pucks show lower stability values. This is likely due to the significant increase in voids present. When above 80° F, the binder in the RAP material appears to be mobilizing, aiding in compaction and also reducing stability due to a reduction in viscosity. This is also consistent with other phases of the research.

Potential for differentiation of the emulsions was looked at. Deviations of the results were most notable below 80 degrees, however it would need to be done with consistent compactive effort. Compactive efforts that are modified to meet a target void content or height will compensate for any variations in the emulsion characteristics.

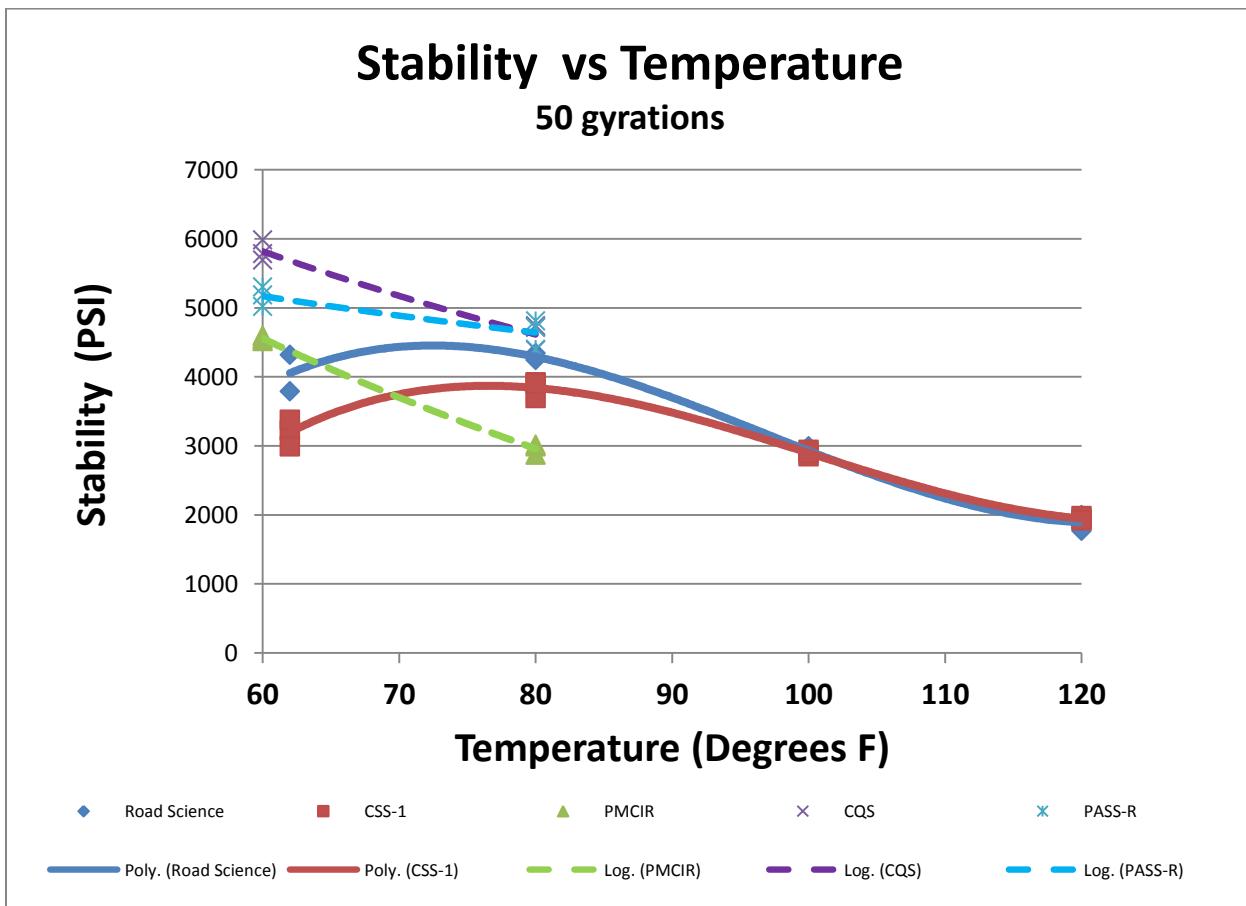


Figure 28: Stability of Samples vs Test Temperature

4.3.2 SAMPLE STABILITY WITH TIME

Observation of the results of the curing time versus stability testing showed that the stabilities were very consistent after about four hours, regardless of the actual length of curing time (Figure 29). The two hour times were significantly higher in stability. Several things are thought to contribute to the increased stability at two hours. This includes the increased compactive effort used to create pucks of the same height as the remaining pucks, the possibility that two hours in the oven is insufficient to reach the 80° F test temperature, and the increased stiffness of the emulsion residue that has been observed in other test evaluations. Subsequent phases of the investigation have used modified procedures where all ingredients are brought to temperature prior to mixing, so as to eliminate any potential thermal gradients.

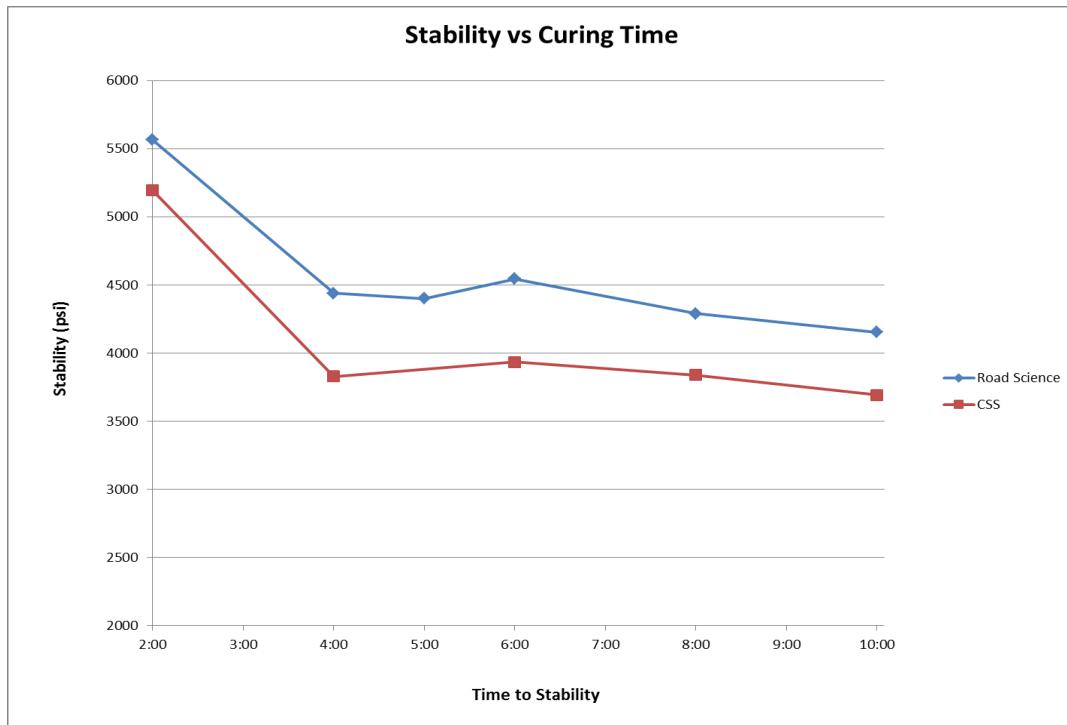


Figure 29: Stability of Samples vs Test Temperature

Results of the testing did not differentiate between emulsions as related to the curing time, however a continual variation in the ultimate stability was present throughout the results, with the CSS-1 exhibiting more than 10% less strength than the Road Science emulsion.

4.4 Data Evaluation Superpave Gyratory Compactor (SGC) AASHTO T 312

4.4.1 RELATIONSHIP BETWEEN DENSITY AND TEMPERATURE

The calculated densities were compared to field densities and the 30 gyration compaction endpoint was selected for plotting against sample height.

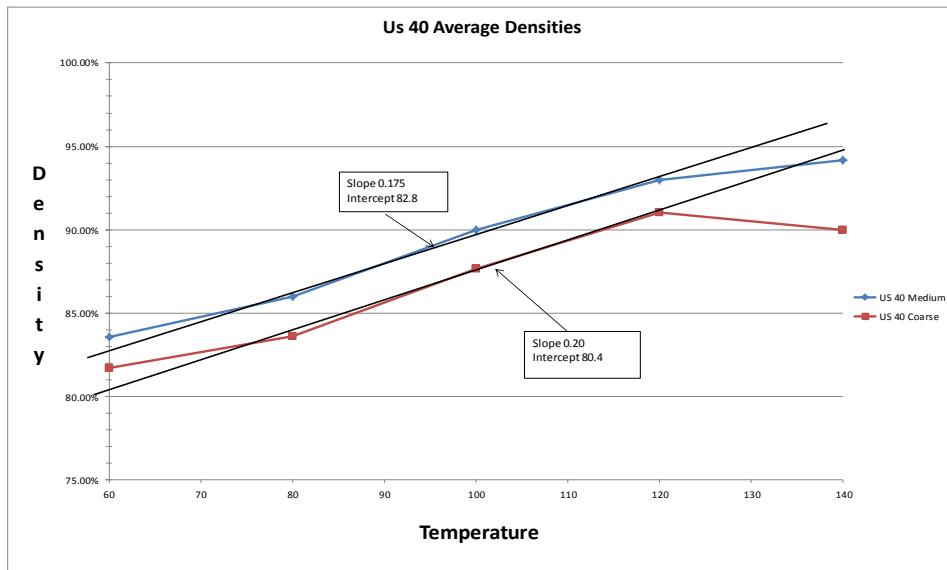


Figure 30: Average Density at 30 Gyration vs Temperature US-40

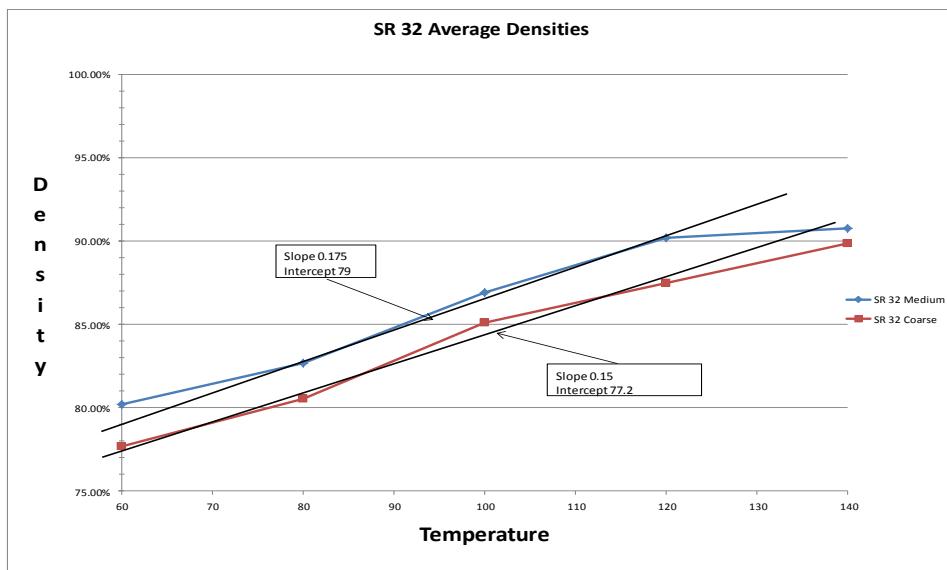


Figure 31: Average Density at 30 Gyration vs Temperature SR-32

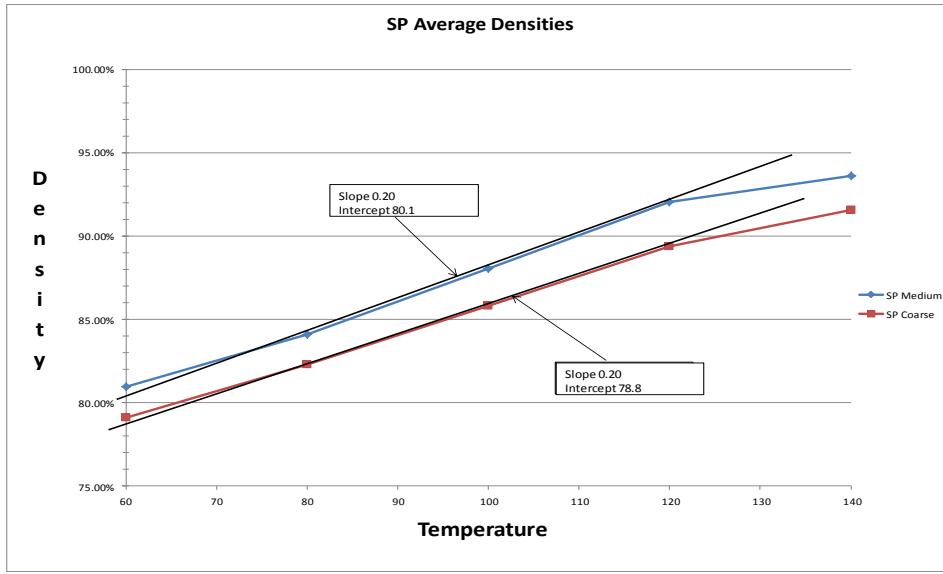


Figure 32: Average Density at 30 Gyration v Temperatures, Southern Parkway

Although each Rap source compacts differently, each of them achieve higher density as temperature rises. That is the endpoint density curve rises as temperature increases. The endpoint density curve is linear between 80°F and 120°F. The slope of these curves falls between 0.15 and 0.20 and could be reasonably estimated at 0.18. Intersection with the density axis is RAP source and gradation dependent however the two sources from wet/freeze climate were more difficult to compact than the dry/no freeze climate.

4.4.2 DENSITY VS GRADATION

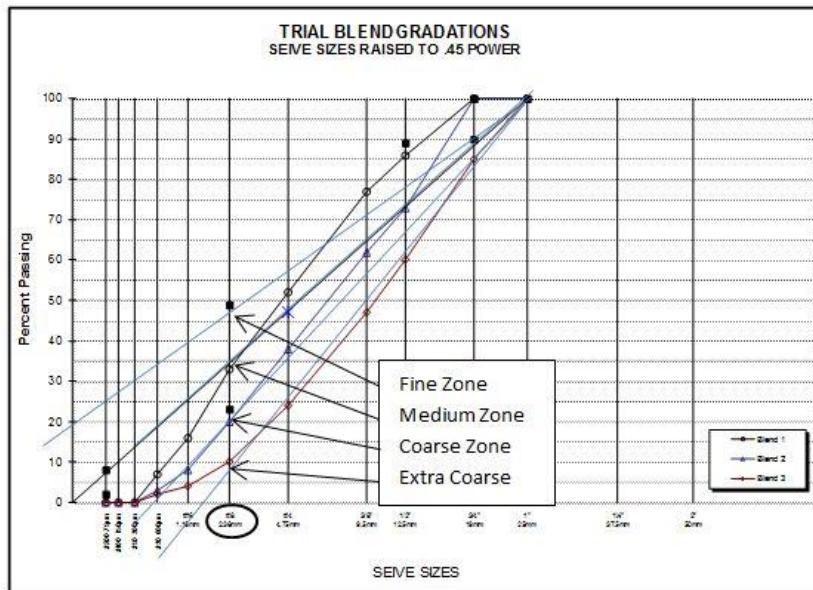


Figure 33: Power 45 Chart with: Blend 1, Medium Gradation

Blend 2, Coarse Gradation

Blend 3, SR-32 Gradation

The density change between medium and coarse gradations is approximately 2% consistently across the measured RAP Sources. Studies have shown that Voids are controlled most significantly by the materials passing the break screen.(usually #8 or #10) The difference between a medium gradation and a coarse gradation in this study is 13% on the #8 screen. Hypothesis: For each 13%-passing change on the #8 screen, there is a 2% change in compacted void at any temperature between 80 and 120°F. This needs to be tested in the field.

4.5 Data Evaluation Semi-circular Bending

The data lends itself to a triangular model where the peak force, the displacement at peak force and the area within the triangle defined by one half the peak force times the displacement at the peak force. These three values are the basis for fracture toughness and fracture energy. They will be shown graphically in Figure 34 to Figure 36.

4.5.1 PEAK FORCE

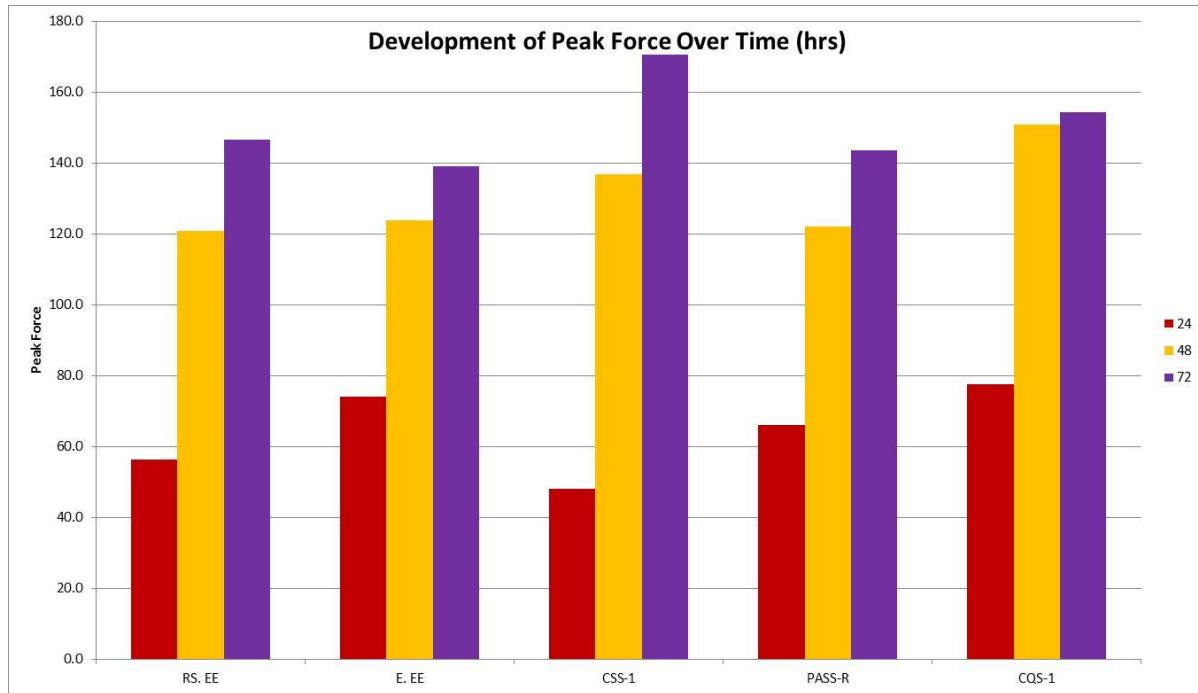


Figure 34: SCB Development of Peak Force over Time

All of the emulsions gain strength over time. The engineered emulsions start high indicating rapid strength gain in the first 24 hours. The CQS1 and Pass are similar. The CSS1 is much slower. Good strength is gain is obtained from all emulsions in 48 hours with the CSS1 and the CQS1 producing higher values at this point. The 72 hour result is very near to the base asphalt performance with the CSS1 showing a harder base while all of the others are similar.

4.5.2 DISPLACEMENT AT PEAK FORCE

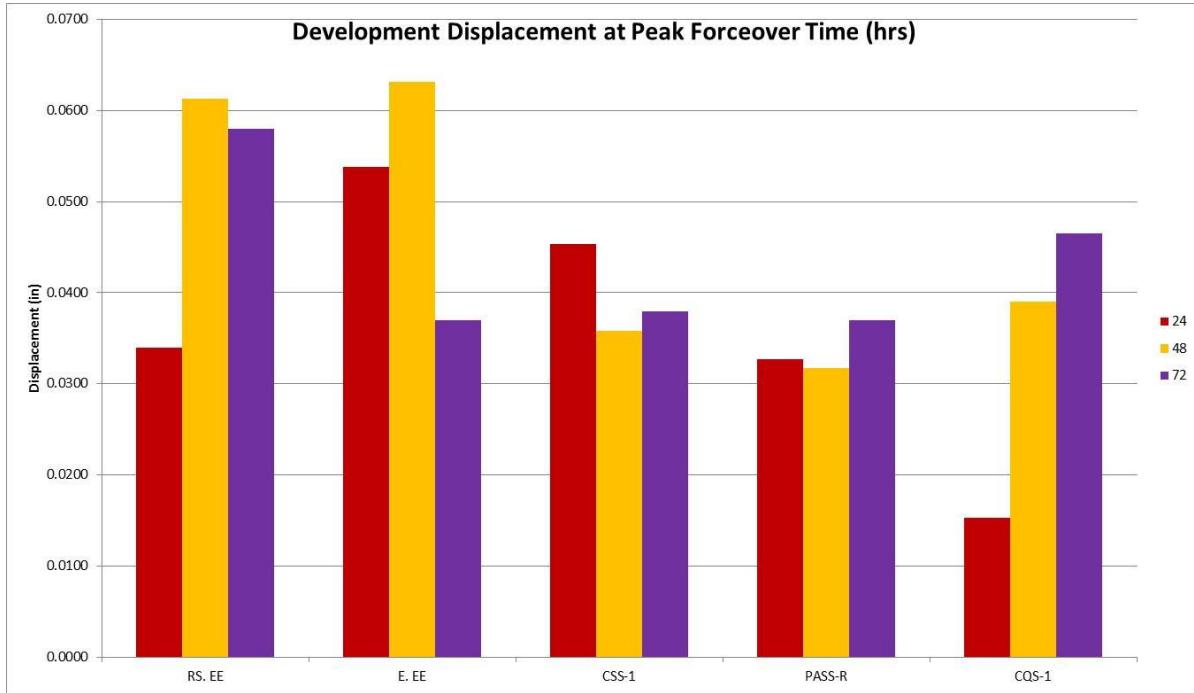


Figure 35: SCB Development of Displacement at Peak Force over Time

Low displacement at peak force is an indication of brittleness or if coupled with low strength, an indication of crack susceptibility. The two engineered emulsions show similar development of ductile strength but the base binders are dissimilar. The CSS1, PassR and CQS all show limited ductility with passing time as compared to the engineered emulsion.

4.5.3 AREA WITHIN THE PEAK FORCE VS DISPLACEMENT AT PEAK FORCE TRIANGLE

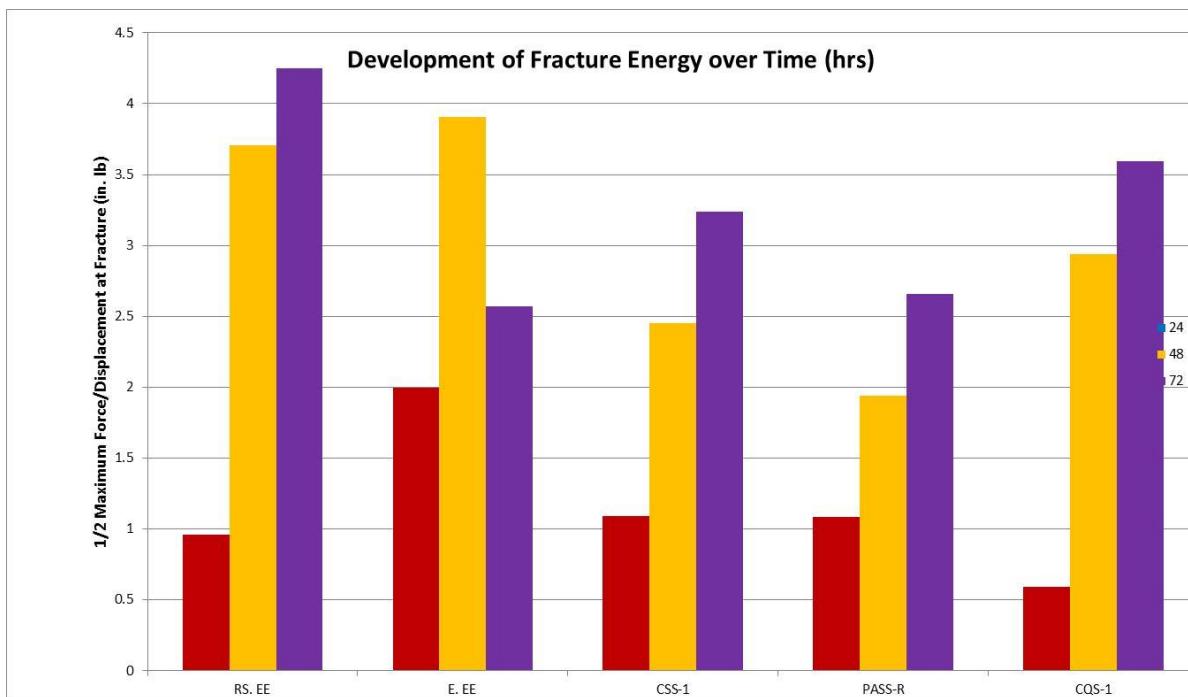


Figure 36: SCB: Development of Area-within-the-Peak-Force vs Displacement-at-Peak-Force Triangle over Time

All of the emulsions exhibit similar but low areas under the force/displacement curve. This indicates low fracture energy early in the mixes' life. Most emulsions exhibit a growing fracture energy over time.

4.5.4 OBSERVATIONS:

All emulsions grow in strength over time. Each is formulated to increase viscosity at a predictable rate. The SCB test has shown an ability to rank emulsion based mixes for intermediate and long term fracture toughness (maximum force) and fracture energy (area under the force/displacement curve). The meaning of the results has yet to be determined.

4.6 Data Forced Cone Penetration

Note that all three emulsions exhibit demulsification onset after 20 minutes. As water is released, viscosity rises and then declines. This decline lasts until 30 to 45 minutes have passed

and then all three emulsions exhibit a sharp increase in viscosity. The EEE and the RSEE indicate a greater percent rise than the ESS1 at the end of 3 hours. This higher viscosity change persists thereafter. It is clear that these are not absolute values. Beginning viscosity is very different in each emulsion. This will be the case when the beginning moisture is different. It has been observed that if evaporation is allowed, break time is reduced and the rate of viscosity gain is increased.

It appears that thresholds can be set for separating CMS, ESCIRE and CSS emulsions. Initial demulsification should result in no more than 30% increase in viscosity at the 30 minute mark and should exhibit an increase greater than 50% at the 3 hour mark.

4.7 Summary

A clear demonstration to RAP densification sensitivity to temperature and gradation has been accomplished. This will lead to the ability to tabulate density targets in mix design. A new test has been developed to measure the rate of demulsification for various emulsion types. This will lead to timing compaction as well as to qualifying asphalt emulsions for use in CIR.

5.0 CONCLUSIONS

5.1 Summary

CIR has proven to be a useful, economical tool in the pavement preservation toolbox. The number of successful projects has been tainted by a few unsuccessful applications. Several issues have come to light during field observation highlighting deficiencies in the current mix design process. Questions regarding density targets, emulsion content gradation control and proper compaction timing are the objects of this research.

5.2 Findings

New understandings of temperature sensitivity and gradation effects on compaction have been gained through the use of Proctor, Marshall and SGC testing. A new test has been developed for evaluating the demulsification of ESCRE as well as other proposed CIR emulsions.

5.2.1 FINDINGS MODIFIED PROCTOR

The materials in use are relatively gap-graded, non-plastic materials. Most of the moisture is held on the particles passing the #8 screen. When these particles reach optimum moisture, compaction ceases. Shortly thereafter, moisture pours into the larger voids and drains from the aggregate structure. A moisture source with higher viscosity (asphalt emulsion) may be made to hang in the larger voids but under modified Proctor compaction energies at room temperatures, density falls off after the fines reach near saturation. These are the compaction conditions used in the existing mix design procedure and are now known to be variable with temperature.

5.2.2 FINDINGS MODIFIED MARSHALL STABILITY

While there does appear to be the potential to perform binder differentiation, the potential would have to be evaluated for variability and sensitivity of results affected by both the timing and temperature and the window for differentiation appears small. Differences in the emulsion

were present at lower temperatures, temperatures that are at the low end of the typical operating spectrum of CIR placement.

Temperatures above 80° F appear to be significantly influenced by the temperature sensitivity of the RAP. The rate of curing of the emulsion is an important factor in the development of strength in the mix however, in addition to temperature and time, other phases of this research have shown that the influence of fines and moisture content, and thereby film-thickness has a very large impact on the curing rate of the mix. This characteristic is discussed later. As part of that discussion, it is noted that the overall compactability of the CIR mix is best performed above 80° F.

5.2.3 FINDINGS SUPERPAVE GYRATORY COMPACTOR

Compactability of RAP is not confined to gradation at room temperature. It is a much more complex issue that changes with base RAP binder and temperature. Since gradation is affected by mill speed which is also affected by temperature, understanding compaction sensitivity to these factors is essential.

The RAP sources used in Utah are made from similar base binder. They all react to temperature in the same way given an offset for aging and original binder stiffness. The various RAPs compact along a line with an 18% slope as temperatures rise between 80° F and 120° F. Below 80° F they behave like an unbound material with discrete particles. Above 120° F the original aggregate structure begins to shape the outcome. The density at 60° F is different for each RAP source and is dependent on the gradation and binder stiffness at this temperature.

A table can now be developed to relate temperature and gradation to target density at the time of compaction. With the aid of a temperature dependent rate-of-demulsification test, target densities can now be determined.

5.2.4 FINDINGS SEMI-CIRCULAR BENDING TEST

Although there is a great deal of discussion regarding the SCB test, CIR seems to be weak and brittle enough in cracking to produce a reasonably linear crack response. Differences between emulsion types, curing time and curing temperature can be observed. It is unknown at

this time whether high fracture toughness and fracture energy is desirable or whether these properties should be limited to a maximum value in a base isolation interlayer. These issues will be resolved with further study.

5.2.5 FINDINGS FORCED CONE PENETRATION TEST

Moisture is held in the RAP gradations according to the fines available. Moisture, including emulsion drains out of the system when the fines become supersaturated. 95% or more of the moisture required for mix supersaturation is contained on the minus #8 screen material while 50% of the moisture is held by the minus #30 fines. The fines must be supersaturated to prevent flash demulsification and a moisture sweep needs to be run above the point of supersaturation to determine whether excess moisture can be used to slow the break. The forced cone penetration test in the configuration proposed can, if sufficiently controlled, differentiate between CSS and ESCIRE emulsions. Experimental thresholds have been set. It is not yet known how the temperature sensitive void space and the supersaturation requirement work together to produce a successful mix.

6.0 RECOMMENDATIONS AND IMPLEMENTATION

6.1 Recommendations

Three outcomes from this research are valuable to the specifier.

1. A tabulation can be developed in mix design which will target emulsion content at any temperature and gradation.
2. A tabulation can be developed so that a density target can be set for any temperature, gradation and RAP source.
3. Emulsions can be evaluated for suitability for use in CIR construction.

6.2 Implementation Plan

Specifications need to be modified to provide for the use of these tools.

REFERENCES

Biligiri, K.P., Said, S., Hakim, H, 2012 “Mixtures’ Crack Propagation Assessment using Semi-Circular Bending Tests” International Journal of Pavement Research Technology

APPENDIX A: Utah Test UT-01-14 Qualifying Engineered Solventless Cold-In-Place Recycling Emulsions (ESCIRE)

Purpose: This test is to determine the rate of demulsibility of an ESCIRE in the presence of water, RAP fines, Hydrated Lime, Constant temperature and vibration. The test will determine viscosity change in the mortar fraction of a RAP mixture over time.

Apparatus: The test requires a vertical press with an adjustable speed control allowing for 0.5 ± 0.05 inches per minute and a load cell with a span of no greater than 200 pounds. A reporting accuracy of 0.1 pound must be provided. The press must have position control with accuracy within 0.01 inch. The data recording device must be capable of recording at a minimum of 50 points per second and must report these points in a form which may be graphed. Use a test head which is 3/4 inches in diameter and 7/8 inches tall with a 90 degree cone coming to an acute tip. The surface of the cone will be finished with a 1000 grit sandpaper with all finish marks being concentric to the axis. Control temperature with an incubator with temperature control to within 1°F. of the target in the range of 60 to 150°F. Vibrate the sample on a vibratory table producing 60 hz. at an amplitude of 0.125 inches.



Figure 37: Test Head

A cup to contain the sample must be correctly sized to reduce interaction between the test head and the sidewalls. A cup at least 2 inches in diameter and 1.75 inches deep is required. A tight fitting lid is required to reduce moisture loss.



Figure 38: Sample Cup

Sample Preparation: Select a test temperature. Samples may be qualified at 80, 100, 120 or 140°F depending on the temperature that processing is expected. Bring all parts of the test sample to test temperature in the incubator. Include mixing tools and sample containers. For the sample cup shown, obtain 900 grams of minus #30 RAP from the proposed project. Obtain 13.5g lime and at least 400g potable water. 258g of emulsion will be added after wetting the aggregate.

Thoroughly mix the RAP and lime with 300g of water. Vibrate the mix for 15 seconds and watch for liquefaction. Increase water content in 10g increments. Mix and vibrate at each increment. Stop when the mortar liquefies. Add 258g emulsion and mix until evenly distributed in the mortar. Consistency should be like a milkshake and should self-level. Spoon the mortar into the sample cups so that each cup contains 80 ± 0.5 g. Cap each sample and vibrate for 15 ± 1 seconds on the vibratory table. Return the samples to the incubator. Increase these weights proportional to the chosen sample container.

Test Procedure: Tests will be run at 10, 20, 30, 45 minutes, 1, 2, 3, 4, 6 and 24 hours. Uncover the sample and place in the test apparatus within one minute of the target time up to one hour and within 5 minutes of the target time after one hour. Gently remove any water from the surface of the sample with a towel. Do not disturb the surface of the mortar. Bring the tip of the test head into contact with the surface of the mortar without penetration. This is the beginning displacement. Press the cone into the mortar at the rate of 0.5 in/min to a depth of 1 inch. Remove the test head from the sample. Clean the head with acetone.

Reporting:

Emulsion Mfgr.

Emulsion Label

Test Technician and Lab

Manufacturer of press and controls system

Date of sample preparation

Time at beginning of each test

Test Temperature

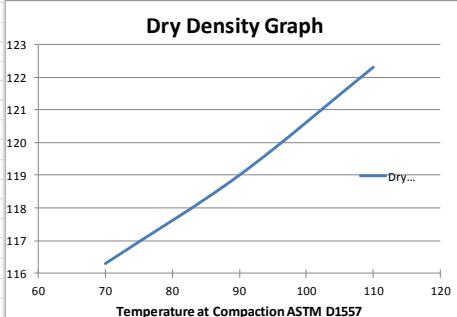
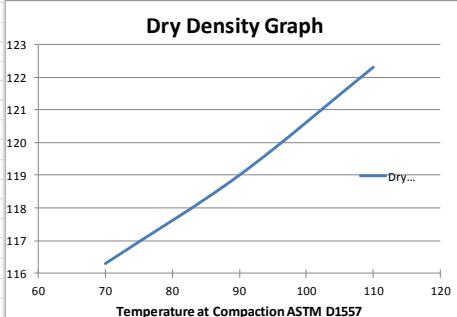
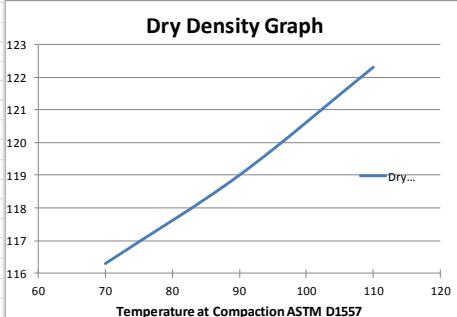
Maximum force required to penetrate 1 inch into the sample at each test time

Graph of maximum force vs time

Graph of % change in force vs time

APPENDIX B: MODIFIED PROCTOR

The Data obtained from the experimental plan using the Modified Proctor is presented here.

		RAP Proctor Densities																																																												
Project	Bluffdale QA		Date	7/2/2014																																																										
Project #	126		Sample location	2700 West on West side of Road at approximately 14000 South- Stockpile																																																										
Test	ASTM D 1557																																																													
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SCB Test Results on CBR Equipment 05-11-14 48 Hour Curing							
050113CQA#1	050113CQA#2	050113CQB#1	050113CQB#2	050113CQC#1	050113CQC#2		
5.4	1.125	6.8	1.173	13.1	1.119	5.3	1.165
5.4	1.125	11.4	1.174	19.7	1.127	5.2	1.165
20	1.134	12.4	1.175	22.1	1.129	5.3	1.165
22.7	1.136	13.6	1.176	24.6	1.13	5.4	1.167
26.4	1.137	15.1	1.177	27.8	1.131	5.4	1.168
30.1	1.138	16.7	1.18	31.5	1.132	5.4	1.168
34.4	1.139	18.9	1.18	35.9	1.134	32	1.175
39.6	1.140	21.8	1.18	41	1.134	36.5	1.176
45.1	1.141	39.0	1.188	46.3	1.134	42.2	1.177
51.5	1.142	43.6	1.188	93.6	1.144	48.8	1.179
58.9	1.144	48.8	1.188	101.2	1.145	64.4	1.179
66.8	1.145	54.7	1.188	109.3	1.146	119.1	1.186
75	1.146	95.9	1.193	117.9	1.147	126.9	1.188
83.7	1.148	104.5	1.193	127.2	1.148	133.7	1.189
92.7	1.148	113.6	1.193	136.5	1.149	138.7	1.19
101.6	1.148	122.9	1.193	145.2	1.15	140.7	1.192
146.2	1.158	152.7	1.2	153.3	1.151	138.9	1.194
149.5	1.159	151.5	1.202	160.6	1.154	137.2	1.195
150.7	1.162	150.4	1.203	166.6	1.155	135.5	1.198
149	1.164	149.2	1.206	171.1	1.156	133.7	1.2
147.2	1.165	148.1	1.208	172.7	1.157	132	1.202
145.5	1.167	146.9	1.21	171	1.159	130.3	1.203
143.8	1.169	145.8	1.212	169.4	1.16	128.5	1.206
142.1	1.172	74.5	1.216	167.7	1.163	52.2	1.208
140.3	1.174	67.9	1.218	166.1	1.165	43.5	1.209
138.6	1.175	62.0	1.22	164.5	1.167	36.8	1.211
83.3	1.176	56.7	1.224	162.8	1.169	31.5	1.212
73.7	1.177	52.0	1.227	161.2	1.172	27.3	1.215
66.7	1.179	48.0	1.231	60.8	1.174	24.3	1.215
60.7	1.180	44.5	1.233	46.1	1.177	22.2	1.215
55.5	1.181	41.3	1.235	35.9	1.18	13.4	1.228
50.6	1.182	39.4	1.238	29.2	1.181	12.4	1.229
45.6	1.190	38.0	1.24	25	1.183	11.5	1.232
39.7	1.191	37.1	1.242	22	1.185	11	1.233
34.3	1.192	36.3	1.244	19.3	1.188	10.8	1.234
32	1.194	35.3	1.246	16.6	1.191	10.4	1.235
30	1.195	34.6	1.249	14.2	1.193	9.8	1.236
28.5	1.197	33.9	1.251	12.5	1.195	9.1	1.237
26.9	1.199	33.2	1.253	11	1.199	8.2	1.238
25.1	1.200	32.6	1.255	9.5	1.202	7	1.238
23.2	1.201	32.1	1.256	8.4	1.207	5.3	1.243
20.8	1.203	31.6	1.259	7.2	1.212	5.2	1.244
18.7	1.205	31.1	1.261	6	1.217	5.1	1.246
16.4	1.207	30.8	1.262	4.9	1.223	5	1.247
14.3	1.208	30.5	1.264	4	1.228	4.9	1.25
13.1	1.209	30.5	1.266	3.3	1.235	4.9	1.252
12	1.211	30.5	1.268	3	1.243	2.8	1.254
11.2	1.212	30.5	1.269	2.5	1.253	2.7	1.255
10.4	1.214	30.5	1.27	2	1.262	2.6	1.256
9.4	1.215	30.5	1.271	1.5	1.267	2.2	1.259

SCB Test Results on CBR Equipment 05-21-14 24 Hour Curing

050213CQA#1	050213CQA#2	050213CQB#1	050213CQB#2	050213CQC#1	050213CQC#2
13.8	1.242	20.5	1.168	10.2	1.208
23.7	1.249	39.9	1.172	11.8	1.208
26.7	1.249	46.1	1.171	27.9	1.215
30.1	1.249	97.3	1.179	30.7	1.216
34.6	1.249	95.1	1.181	34.8	1.216
53.4	1.255	92.9	1.182	39.4	1.216
53.9	1.256	36.6	1.184	77.1	1.22
54.5	1.256	31.5	1.185	83.7	1.221
55.1	1.256	27.4	1.188	89.8	1.223
33.1	1.26	24.1	1.189	95.4	1.224
29.9	1.262	21.3	1.19	104.3	1.224
27.6	1.263	18.9	1.192	102.9	1.235
26.4	1.266	16.8	1.193	102.3	1.237
25.4	1.268	14.8	1.195	101.6	1.24
24.4	1.27	12.9	1.198	29.5	1.241
23.1	1.273	11.4	1.199	25	1.243
21.7	1.278	10.2	1.2	21.4	1.245
19.8	1.281	9.1	1.201	19	1.247
17.9	1.287	8.4	1.203	17	1.25
16.4	1.29	7.5	1.205	15	1.252
14.9	1.296	6.3	1.207	13.2	1.253
13.4	1.301	5.6	1.208	11.5	1.254
11.9	1.306	4.8	1.21	10.2	1.255
10.5	1.311	3.5	1.211	9.1	1.256
9	1.315	2.7	1.214	8.1	1.258
7.7	1.32	2.2	1.216	7.4	1.259
					6.2
					6.1
					6
					5.9
					3
					2.8
					2.8
					2.8
					2.8
					2.8
					2.8
					2.8
					1.9
					1.8
					1.8
					1.8
					1.8
					1.8
					1.9
					1.9

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APPENDIX F: FORCED CONE PENETRATION

The Data obtained from the experimental plan using the Forced Cone Penetration is presented here.

Force at 0.75" Penetration			
Minutes	EEE	CSS1	RSEE
10	1.01	3.14	3.7
20	1.75	3.11	
30	1.38	3.98	4.8
45	1.63	3.55	
60	2.93	3.81	4.32
120	2.47	4.02	5.22
180	2.79	5.96	8.2
240	3.34	5.99	
360	4.79	8.04	
1440	3.42	7.12	

Table 14: Force at 0.75" at Various Cure Times